

# **TRAFFIC IMPACT ASSESSMENT**

IN SUPPORT PLANNING PROPOSAL

PROPOSAL TO AMEND THE ORANGE LOCAL ENVIRONMENTAL PLAN 2011  
IN RESPECT OF LAND AT LEEDS PARADE, CLERGATE

PREPARED FOR:

**ROSEDALE GARDENS ESTATE PTY LTD**

AUGUST 2019



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The preparation of this report has been in accordance with the project brief provided by the client and has relied upon the information, data and results provided or collected from the sources and under the conditions outlined in the report.

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*T&TPA Traffic Report*

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# Introduction

## 1.1 BACKGROUND AND PROPOSED DEVELOPMENT

Rosedale Gardens Estate Pty Ltd intends to develop a staged large lot residential subdivision on land consisting of the former Orange abattoir at the northern end of Leeds Parade, Orange.

In order to facilitate the subdivision an amendment to the Orange Local Environmental Plan 2011 is required to rezone the subject land from RU1 – Primary Production and IN1 – General Industrial to R5 – Large Lot Residential and E4 – Environmental Living.

The subject land is described in **Section 1.2** and has an overall area of approximately 290 hectares.

The subject land would be developed with internal local roads and these would be accessed from Leeds Parade in the south. At this time no further connections to existing roads are proposed however it anticipated that development of adjacent land in the future would facilitate an alternate access to Ophir Road. This does not however form part of this assessment. Additionally, an access to Pearce Lane has been considered but discounted at this time due to the costs associated with upgrading Pearce Lane and the Pearce Lane/Clergate Road and Clergate Road/Northern Distributor Road intersections.

A master plan for the site has been developed generating approximately 450 lots with a minimum size of 4,000 square metres.

A staging plan is predicated on the development commencing at the Leeds Parade intersection (south) and the release of 10-20 lots per year, to account for demand requirements, without over saturation.

This assessment is conducted on the development at completion, that is, the impact of vehicles generated by all 450 lots on the operation of the local road system.

## 1.2 SUBJECT SITE

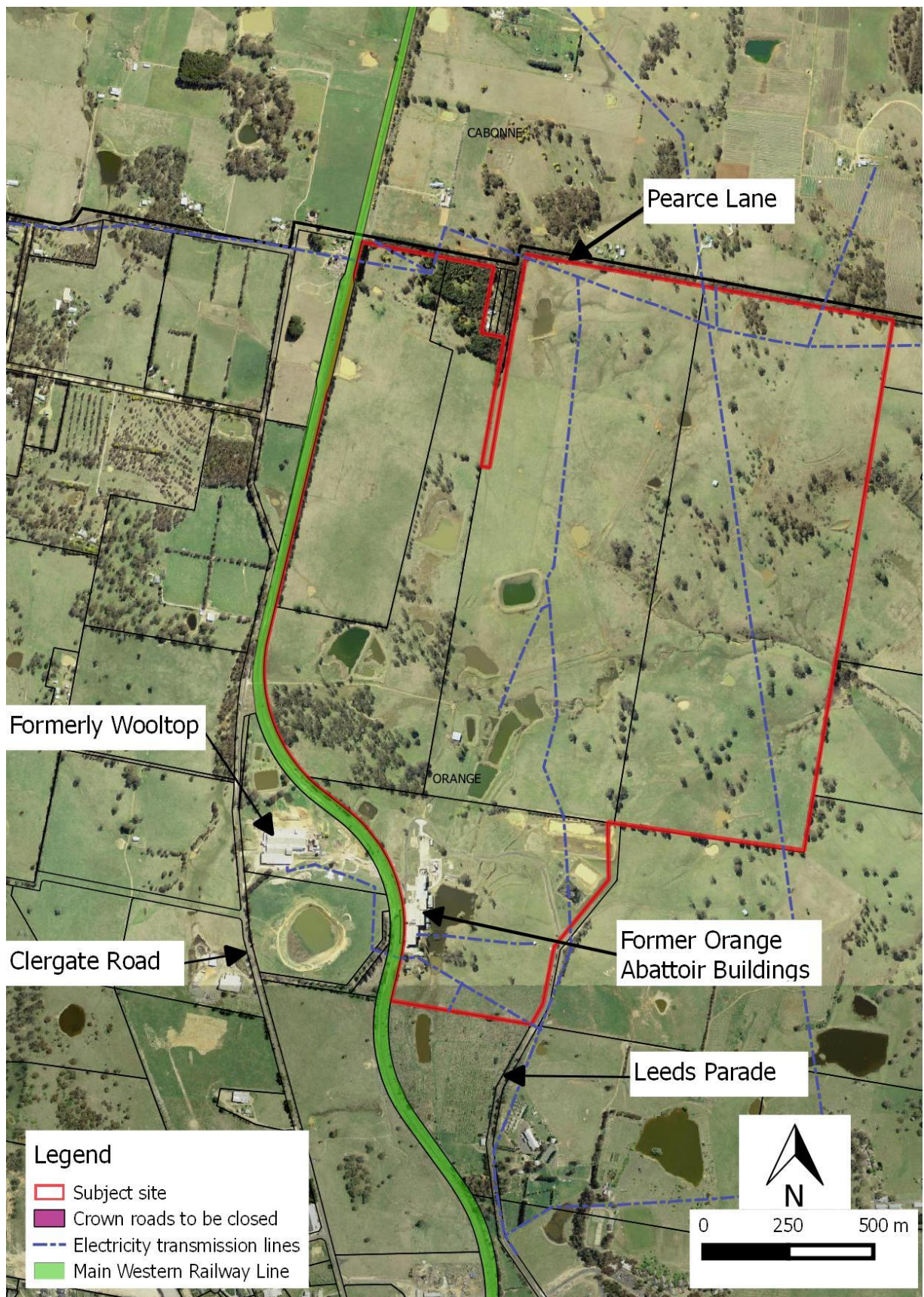
The subject site is formed of:

- Lot 15 DP6694, 390 Clergate Road, Orange
- Lot 3 DP255983, 440 Clergate Road, Orange
- Lot 2 DP255983, 440 Clergate Road, Orange
- Lot 14 DP6694, 440 Clergate Road, Orange
- Lot 25 DP6694, 440 Clergate Road, Orange

The site is located approximately 5 kilometres north of Orange central business district (CBD) and 1.8 kilometres (3.5 kilometres by road) from the North Orange shopping centre. The site has an area of approximately 290 hectares and is bounded to the north by Pearce Lane, to the west by the Main Western Railway Line and to the south and east by private late. The southern portion of the site (Lot 15) is currently zoned IN1 – General Industrial with the remainder of the currently zoned RU1 – Primary Production.

The site is depicted in **Figure 1**.





## 1.3 METHODOLOGY

By reference to clause 104 of the *State Environmental Planning Policy (Infrastructure) 2007* (ISEPP), the resulting proposed subdivision would be classified as a Schedule 1 traffic generating development in the basis that it proposes 300 or more lots. The matter is therefore referable to the Roads and Maritime Services and a Traffic Impact Assessment (TIA) has been prepared to assist the development assessment process.

The proposal to rezone the land would result in the potential for the development of a large lot residential subdivision. This eventual subdivision would generate an increase in the volume of traffic movements onto the surrounding road network. The provision of safe and efficient means of access to the development will be necessary to ensure the satisfactory operation of the development.

This Traffic Impact Assessment (TIA) investigates the impact of the development on the surrounding road network. The TIA will determine if additional traffic management facilities are required to safely and efficiently control the movement of all vehicular traffic to and from the site.

This TIA will address the following specific issues:

- The potential levels of traffic generation as a result of the proposed rezoning;
- The capacity of the affected roads to accommodate additional traffic;
- Impact on safety and amenity of the surrounding road network; and
- Operation of specific affected intersections.

The methodology for preparing the TIA is outlined below:

- i. Review of existing traffic volume data held by Council and available from external sources for roads surrounding the development site.
- ii. Determination of the traffic generating potential of the rural residential subdivision development and calculation of the peak hour and peak daily traffic volumes to be added to the existing traffic volumes on the roads surrounding the development site.
- iii. Determination/collation of traffic generation data for surrounding traffic generating developments (notably, the Bunnings hardware store and highway service centre both currently under development on Leeds Parade);
- iv. Assessment of the impact of the additional traffic generated by the subdivision development on the surrounding road network through the use of SIDRA modelling, including the impacts associated with traffic generated by development in the surrounding area. The impact assessment will be carried out in terms of:
  - Road capacity; and
  - Level of Service.

In summary, this Traffic Impact Assessment will assess:

- the existing traffic movements on the existing road network, including Northern Distributor Road (both east and west of Leeds Parade) and Leeds Parade (both north and south of Northern Distributor Road);
- the expected traffic volumes generated by the proposed subdivision;
- the effect of the generated traffic on the surrounding roads; and
- review of the operation and design of existing intersections to determine whether anticipated volumes can be accommodated.



# Existing Traffic Conditions

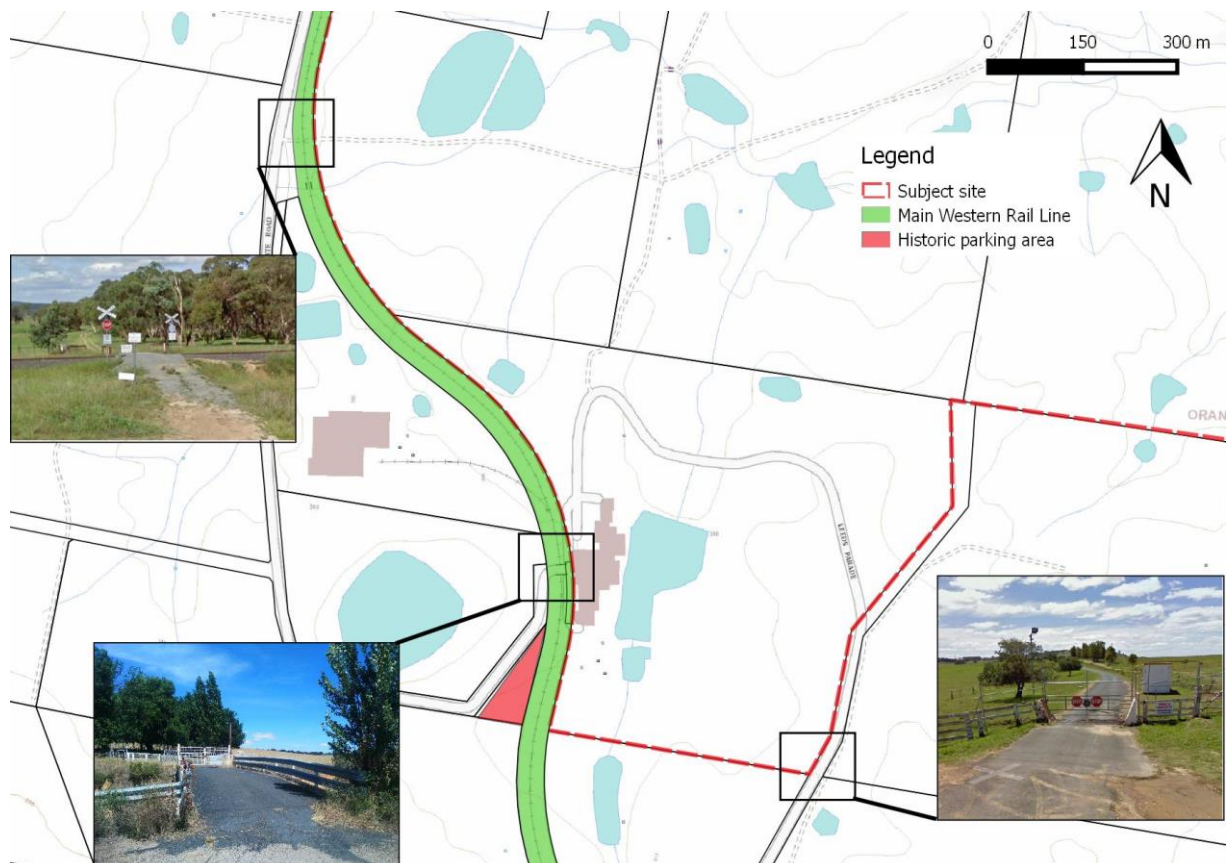
## 2.1 INTRODUCTION

Primary access to the site is from Leeds Parade in the south, with Leeds Parade transitioning into the sealed driveway that formerly provided heavy vehicle access to the abattoir buildings.

Historically abattoir staff are understood to have accessed the abattoir via a single lane bridge over the Main Western Railway Line, which staff used to walk over after parking on the land on the western side of the rail line. The bridge is accessed from private land owned by the applicant but not forming part of this application.

The site also currently benefits from a single lane rail crossing from Clergate Road and a number of gateway accesses to Pearce Lane in the north.

**Figure 2** shows the south-western corner of the site and the three current (known) accesses into the property in this area.



**Figure 2: Existing site accesses in the southern extent**

The connectivity provided by the alignment of Leeds Parade to Northern Distributor Road affords excellent links to the Orange CBD via Leeds Parade South, Bathurst and Sydney via Northern Distributor Road east and the North Orange Shopping Centre and onwards to Wellington, Dubbo, Parkes, Forbes and Cowra (and south to Melbourne) via Northern Distributor Road west.



As outlined in **Section 1.1** the design intention is to utilise Leeds Parade for all traffic into and out of the subdivision. Access to the site will be via an extension of Leeds Parade with a suitable gateway treatment to indicate the entry to the proposed subdivision. To assess the impact of the proposed subdivision on the surrounding road network the following roads and intersections will be studied:

- Leeds Parade – north of Northern Distributor Road;
- Leeds Parade – south of Northern Distributor Road;
- Northern Distributor Road – east of Leeds Parade.
- Northern Distributor Road – west of Leeds Parade.
- The Northern Distributor Road/Leeds Parade Intersection
- The University Access Intersection

## **2.2 ROAD NETWORK HIERARCHY**

The Roads and Traffic Authority (2008) proposes four basic road classes as the basis for the functional hierarchy of a road network.

Functional classifications take into account the relative balance of the traffic mobility function and amenity/access functions of streets and roads and defines the purpose of a road within the context of an urban area.

The four road classes are motorways, arterial, sub-arterial and local roads and are defined below.

### ***Motorways***

This is the highest form of arterial road and is considered separately due primarily to traffic function and strict access control via grade separate interchanges. These roads provide for major inter-regional traffic movements in a safe and operationally efficient manner.

### ***Arterial Roads***

Roads whose main function is to carry through traffic from one region to another forming the principal means of communication for major traffic movements. Access to land should be limited.

### ***Sub-Arterial Roads***

Those roads which supplement the arterial roads in providing for through traffic movement to an individually determined limit that is sensitive to both roadway characteristics and adjoining land uses.

### ***Local Roads***

Roads that distribute traffic between the arterial roads and the local street system and provide access to adjoining property.

Based on the existing road network the functional classification of Leeds Parade and Northern Distributor Road (in the vicinity of the subject site) are:

Leeds Parade - south of NDR and from NDR to the University access:	Sub-Arterial Road.
Leeds Parade - north of the University access:	Local Road.
Northern Distributor Road:	Sub-Arterial Road.

## **2.3 EXISTING ROADWAY CONDITIONS**

### **2.3.1 LEEDS PARADE**

Leeds Parade is a two way two lane bitumen sealed road with no kerb and guttering.

From the subject site to the University entrance Leeds Parade is approximately 6-6.5m wide with no line markings. The post speed limit is 50 km/hr

From the University Entrance to Northern Distributor Road, Leeds Parade is approximately 6.5–7.5m wide with line marked lanes approximately 3.2 m wide. The posted speed limit is 80 km/hr from Northern Distributor Road to approximately 200m south of the University Entrance where the posted speed limit drops down to 50 km/hr.

South of Northern Distributor Road, Leeds Parade is a two lane, two way road with line marking and a speed limit of 80km/hr for a distance of approximately 650 metres south of the Northern Distributor Road intersection, at which point the speed limit reduces to 50km/hr as it approaches residential and industrial areas of Orange.

There is a bicycle/pedestrian pathway running along the western side of Leeds Parade starting at the University entrance and running south to link into a path on Northern Distributor Road and then continuing south to link into the residential areas of Orange.

### **2.3.2 NORTHERN DISTRIBUTOR ROAD.**

Northern Distributor Road is a two way, two lane bitumen sealed road. Northern Distributor Road has full line marking with average lane widths of 3.7m and sealed shoulders of variable widths

East of Leeds Parade the posted speed limit on Northern Distributor Road is 80 km/hr whilst the remainder has a posted speed limit of 70 km/hr.

There is a bicycle/pedestrian pathway on the southern side of Northern Distributor Road (west of Leeds Parade) that links up to the bicycle/pedestrian pathway in Leeds Parade and into the general urban bicycle/pedestrian pathway network to the west.

Northern Distributor Road acts as a distributor for traffic looking to access the areas of West Orange and North Orange, including the North Orange shopping centre, and the areas of East Orange, including homemaker centre, and onwards to Bathurst and Sydney.

## **2.4 EXISTING ROAD CAPACITY**

Roads have varying capacities dependent on the function they are performing. The United States Highway Capacity Manual defines capacity as follows:

*...the maximum hourly rate at which persons or vehicles can reasonably be expected to traverse a point or uniform section of a lane or roadway during a given time period under the prevailing roadway, traffic and control conditions.*

The physical characteristics of a roadway such as lane width, alignment, frequency of intersections etc. make up the prevailing roadway conditions.

Based upon its capacity and a driver's expectations of the operational characteristics of a traffic stream is a qualitative measure denoted as the level of service of a road.

Level of service definitions combine such factors as speed, travel time, safety, convenience and traffic interruptions and fall into six levels of service categories ranging from A down to F.

The *AUSTROADS Guide to Traffic Management, Part 3 – Traffic Studies and Analysis* describes Level of Service A as:

*A condition of free flow in which individual drivers are virtually unaffected by the presence of others in the traffic stream. Freedom to select desired speeds and to manoeuvre within the traffic stream is extremely high, and the general level of comfort and convenience provided is excellent.*

The categories are graduated from Level of Service A down through six levels to Level of Service F, being a zone of forced flow. The amount of traffic approaching the point under consideration exceeds that which can pass it. Flow breakdowns occur and queuing and delays result.

The *AUSTROADS Guide to Traffic Management, Part 3 – Traffic Studies and Analysis* sets out typical mid-block capacities for various types of urban road with interrupted flow. Based on the physical configuration of Leeds Parade and Northern Distributor Road together with the rates given in the Guide the capacity of the surrounding roads can be determined as:

- Leeds Parade – Capacity of 900 vehicles per hour per lane;
- Northern Distributor Road - Capacity of 1,200 vehicles per hour per lane.

## **2.5 EXISTING TRAFFIC**

Site specific traffic data was not collected on individual roads surrounding the subdivision site for the preparation of this TIA. However, Geolyse has been able to access traffic data from the Traffic Impact Assessment prepared by Transport and Traffic Planning Associates (T&TPA) for the approved Orange Bunnings development, located on the corner of Leeds Parade and Northern Distributor Road. This report provides traffic data for both existing and post development conditions following the development of the proposed Bunnings development. A copy of this report is included in **Appendix A**.

Based on the report by T&TPA, heavy vehicles account for approximately 5% of traffic on Leeds Parade and Northern Distributor Road. These figures are used for modelling purposes across all traffic movements with the exception of Hanrahan Place (Hanrahan Place is a newly constructed fifth leg off the Northern Distributor Road/Leeds Parade roundabout that provides access to the Finemore Depot and for the purposes of the existing traffic scenario it has been assumed that 100% of vehicles on this leg are heavy).

### **2.5.1 ANNUAL AVERAGE DAILY TRAFFIC**

Annual Average Daily Traffic (AADT) is defined as the total volume of traffic passing a roadside observation point over a period of a year divided by the number of days in a year.

AADT data on Leeds Parade north is not available, however it has been predicted based on the peak hour figures provided within the T&TPA report. Assuming the peak hour traffic (PHT) volume represents 10% of the annual average daily traffic (AADT) (based on the ratio of PHT to AADT for surrounding roads), the peak hour traffic can be calculated as:

$$\begin{aligned}\text{Annual Average Daily Traffic} &= 111/10\% \\ &= 1,110 \text{ vehicles/day}\end{aligned}$$

Existing AADT for Northern Distributor Road and Leeds Parade south was obtained from the T&TPA report and **Table 2.1** contains a summary of the AADT on the roads surrounding the subject site.

**Table 2.1 – Existing Annual Average Daily Traffic (AADT)**

Road Location	AADT (vehicles/day)
Northern Distributor Road	8,400
Leeds Parade (south of Northern Distributor Road)	3,500
Leeds Parade (north of Northern Distributor Road)	1,110

Source: T&TPA Report – Refer Appendix A

## 2.5.2 PEAK HOUR TRAFFIC

The T&TPA report contains existing peak hour traffic for both the weekday PM and the weekend midday peaks based on traffic counts undertaken at the roundabout at the intersection of Leeds Parade and Northern Distributor Road. The weekday PM peak occurred at 16:15 to 17:15 and was greater than the weekend midday peak and hence will be adopted as the critical time period for peak hour traffic assessment for the roundabout and surrounding roads.

A summary of the peak hour traffic on the roads surrounding the subject site is contained in **Table 2.2**.

**Table 2.2 – Existing Peak Hour Traffic**

Road Location	Lane	Weekday PM Peak Hour Traffic (vehicles/hour)
Northern Distributor Road (east of Leeds Parade)	Eastbound	241
	Westbound	321
<b>TOTAL</b>		<b>562</b>
Northern Distributor Road (west of Leeds Parade)	Eastbound	362
	Westbound	580
<b>TOTAL</b>		<b>942</b>
Leeds Parade (north of Northern Distributor Road)	Northbound	28
	Southbound	83
<b>TOTAL</b>		<b>111</b>
Leeds Parade (south of Northern Distributor Road)	Northbound	322
	Southbound	242
<b>TOTAL</b>		<b>564</b>

Source: T&TPA Report – Refer Appendix A

## 2.6 EXISTING INTERSECTION CONDITIONS

### 2.6.1 INTRODUCTION

Intersections are critical points which often control the capacity of the road network. This is due to the need for conflicting traffic movements to share the same road space at these locations. The operation of the intersections in the vicinity of the site have been analysed using SIDRA Intersection. SIDRA Intersection is a computerised traffic evaluation tool which is used in the assessment and design of intersection treatments in terms of capacity and operation. The program provides outputs which include delays, queue lengths, total capacities, travel times, and average speeds. SIDRA Intersection is the preferred intersection analysis tool of Roads and Maritime and many Local Government Authorities.



As part of the intersection analysis, the SIDRA Intersection model provides an “Average Control Delay” for each approach of the intersection. This control delay is the average delay per vehicle over the peak hour as a result of the intersection and is grouped into six bands labelled A through to F (with ‘A’ being best and ‘F’ being over capacity), termed Level of Service (LOS). The overall LOS for a ‘Give Way’ intersection is the LOS result for the worst case turning movement.

## **2.6.2 LEEDS PARADE AND NORTHERN DISTRIBUTOR ROAD**

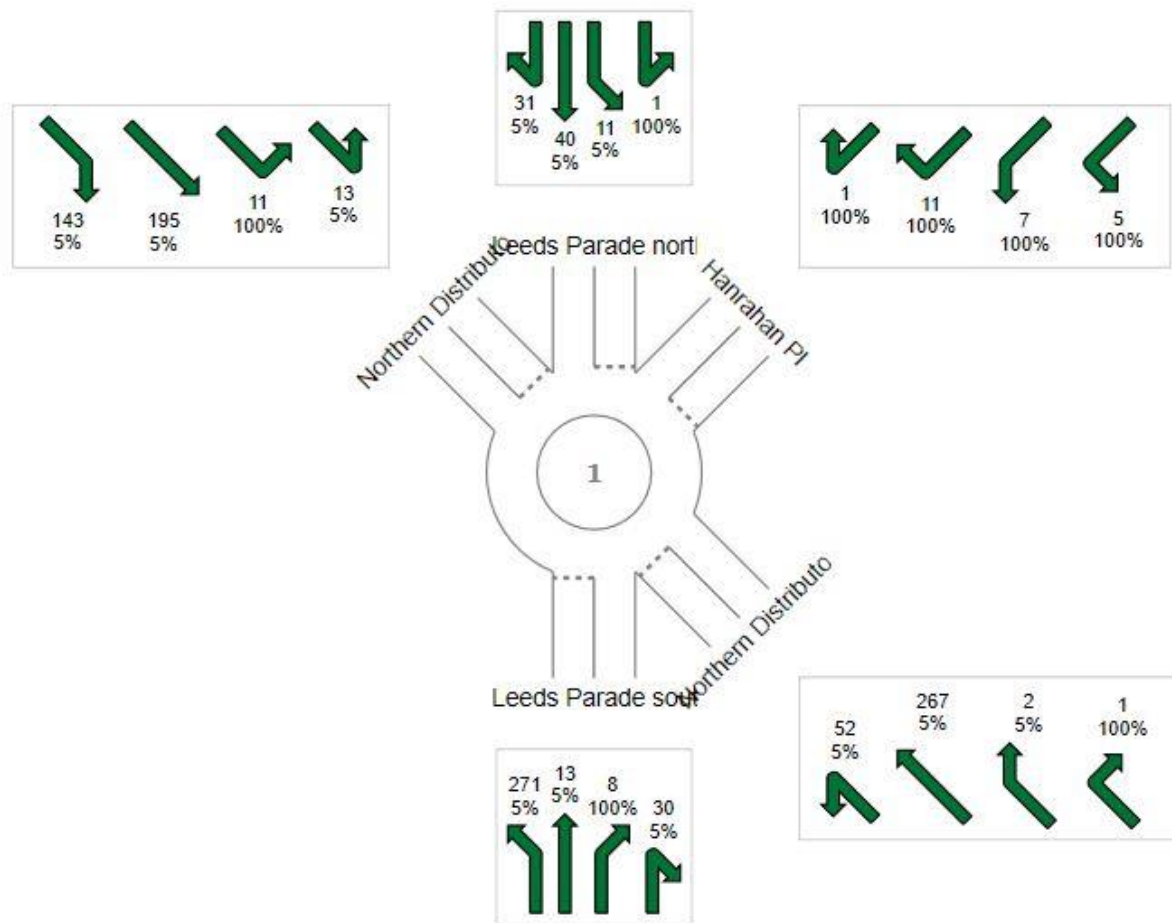
The intersection of Leeds Parade and Northern Distributor Road features a two lane roundabout for all directions with a 36 metre diameter island.

An assessment of the Leeds Parade/Northern Distributor Road intersection under existing weekday PM peak hour conditions was undertaken using SIDRA.

The following assumptions were included in the SIDRA analysis:

- Intersection traffic and turning movements: refer **Figure 3**
- Heavy Vehicle percentage refer **Figure 3**
- Control type: Give Way – all legs
- Lane widths assumed: 3.7m – all lanes
- Approach lengths on Northern Distributor Road
  - 500m westbound through (120m inside lane)
  - 500m eastbound through (60m inside lane + 20m left turn slip lane)
- Approach lengths on Leeds Parade
  - 500m southbound through (100m inside lane)
  - 500m northbound through (60m inside lane)
- Approach lengths on Hanrahan Place – 190m
- Approach and exit cruise speeds:
  - Northern Distributor Road east 70 kph
  - Northern Distributor Road west 80 kph
  - Leeds Parade (north and south) 80 kph
  - Hanrahan Place 50 kph

Based on the above assumptions, a SIDRA Intersection analysis was carried out for the intersection for the existing weekday PM peak. The movement summary output from the SIDRA assessment is shown in **Figure 4**. Full results of the SIDRA analysis are provided in **Appendix B**.



**Figure 3: NDR/Leeds Prd Int – Existing weekday PM Peak hour traffic volumes**

## MOVEMENT SUMMARY

Site: WD PM (NDR/Leeds) - existing

New Site  
Roundabout

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Leeds Parade south											
1a	L1	271	5.0	0.217	9.8	LOS A	1.0	7.4	0.41	1.19	58.8
2	T1	13	5.0	0.072	22.3	LOS B	0.3	2.3	0.45	1.50	51.2
3a	R1	8	100.0	0.072	22.3	LOS B	0.3	2.3	0.45	1.50	51.2
3b	R3	30	5.0	0.072	22.3	LOS B	0.3	2.3	0.45	1.50	51.2
Approach		322	7.4	0.217	11.8	LOS A	1.0	7.4	0.42	0.62	57.2
SouthEast: Northern Distributor east											
21b	L3	52	5.0	0.032	15.1	LOS B	0.1	1.0	0.26	1.28	58.6
22	T1	267	5.0	0.093	9.7	LOS A	0.4	3.0	0.32	1.10	59.5
23a	R1	2	5.0	0.093	10.0	LOS A	0.4	2.9	0.33	1.12	59.2
23	R2	1	100.0	0.093	10.0	LOS A	0.4	2.9	0.33	1.12	59.2
Approach		322	5.3	0.093	10.5	LOS A	0.4	3.0	0.31	0.56	59.3
NorthEast: Hanrahan PI											
24	L2	5	100.0	0.042	37.8	LOS C	0.2	2.0	0.50	1.48	49.2
24a	L1	7	100.0	0.042	37.8	LOS C	0.2	2.0	0.50	1.48	49.2
26	R2	11	100.0	0.042	37.8	LOS C	0.2	2.0	0.50	1.48	49.2
26b	R3	1	100.0	0.042	37.8	LOS C	0.2	2.0	0.50	1.48	49.2
Approach		24	100.0	0.042	37.8	LOS C	0.2	2.0	0.50	0.74	49.2
North: Leeds Parade north											
7b	L3	1	100.0	0.037	12.8	LOS A	0.1	1.1	0.39	1.23	59.7
7a	L1	11	5.0	0.037	12.8	LOS A	0.1	1.1	0.39	1.23	59.7
8	T1	40	5.0	0.037	14.0	LOS A	0.1	1.1	0.40	1.27	58.1
9b	R3	31	5.0	0.037	19.0	LOS B	0.1	1.1	0.41	1.46	52.2
Approach		83	6.1	0.037	15.7	LOS B	0.1	1.1	0.40	0.67	56.0
NorthWest: Northern Distributor west											
27b	L3	13	5.0	0.128	11.0	LOS A	0.6	4.9	0.18	1.07	57.6
27	L2	11	100.0	0.128	11.0	LOS A	0.6	4.9	0.18	1.07	57.6
28	T1	195	5.0	0.128	12.1	LOS A	0.6	4.9	0.18	1.12	56.8
29a	R1	143	5.0	0.128	17.5	LOS B	0.6	4.7	0.19	1.37	53.4
Approach		362	7.9	0.128	14.1	LOS A	0.6	4.9	0.19	0.61	55.4
All Vehicles		1113	8.8	0.217	14.8	LOS B	1.0	7.4	0.31	0.60	56.3

Level of Service (LOS) Method: Delay (RTA NSW).  
 Vehicle movement LOS values are based on average delay per movement  
 Intersection and Approach LOS values are based on average delay for all vehicle movements.  
 Roundabout Capacity Model: SIDRA Standard.  
 SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.  
 Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).  
 HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

**Figure 4: NDR/Leeds Prd Int – Existing weekday PM Peak hour traffic movements**

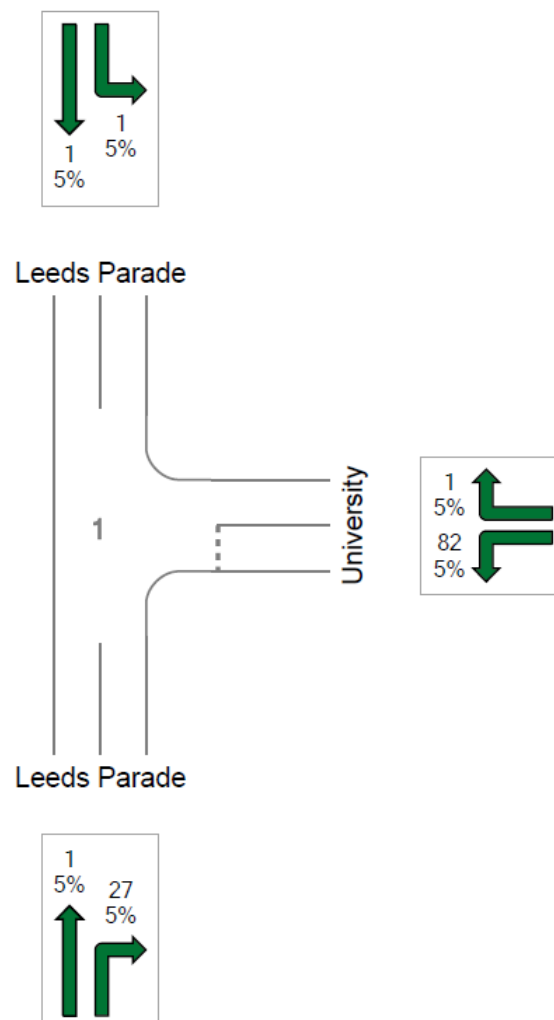
From the table in **Figure 4**, it can be seen that the current worst movement is for those vehicles exiting Hanrahan Place, with a LOS C and an average delay of 37.8 seconds. All other movements operate at a LOS A or B.

### 2.6.3 UNIVERSITY ENTRANCE

The intersection at the entrance to the University consists of a channelised right turn treatment with raised concrete medians.

An assessment of the Leeds Parade/University Entrance intersection under existing weekday PM peak hour conditions was undertaken using SIDRA. The following assumptions were included in the SIDRA analysis:

- Intersection traffic and turning movements: refer **Figures 5** (It was assumed only 1 vehicle per hour travelled past the University with all other traffic entering and leaving the University)
- Heavy Vehicle percentage refer **Figure 5**
- Control type: Give Way – University to Leeds Parade
- Lane widths assumed: 3m – all lanes
- Approach lengths on Leeds Parade
  - 500m northbound through (50m right turn lane)
- Approach length on University Entrance – 140m
- Approach and exit cruise speeds:
  - 50kph all roads



**Figure 5: University Ent Int – Existing weekday PM Peak hour traffic volumes**

Based on the above assumptions, a SIDRA Intersection analysis was carried out for the intersection for the existing weekday PM peak. The movement summary output from the SIDRA assessment is shown in **Figure 6**. Full results of the SIDRA analysis are provided in **Appendix B**.



## MOVEMENT SUMMARY

Site: Uni existing PM

New Site

Giveaway / Yield (Two-Way)

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Flows Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Leeds Parade											
2	T1	1	5.0	0.001	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
3	R2	27	5.0	0.015	8.0	LOS A	0.1	0.5	0.02	0.67	42.8
Approach		28	5.0	0.015	7.7	NA	0.1	0.5	0.02	0.65	43.0
East: University											
4	L2	82	5.0	0.052	7.5	LOS A	0.2	1.6	0.01	0.63	43.1
6	R2	1	5.0	0.052	7.5	LOS A	0.2	1.6	0.01	0.63	43.1
Approach		83	5.0	0.052	7.5	LOS A	0.2	1.6	0.01	0.63	43.1
North: Leeds Parade											
7	L2	1	5.0	0.001	3.7	LOS A	0.0	0.0	0.00	0.40	46.3
8	T1	1	5.0	0.001	3.7	LOS A	0.0	0.0	0.00	0.40	46.3
Approach		2	5.0	0.001	3.7	NA	0.0	0.0	0.00	0.40	46.3
All Vehicles		113	5.0	0.052	7.5	NA	0.2	1.6	0.01	0.63	43.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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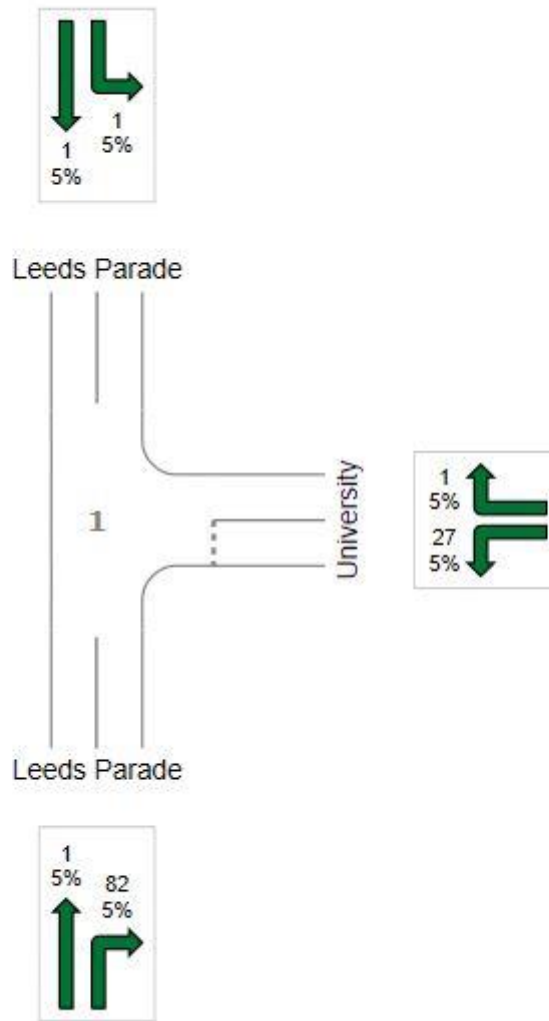
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**SIDRA  
INTERSECTION 6**

**Figure 6: University Ent Int - Existing weekday PM Peak hour traffic movements**

From the table in **Figure 6**, it can be seen that all movements operate at a LOS A, with the worst average delay of 8.0 seconds for the right turn movement off Leeds Parade into the University.

In assessing the performance of the University entrance intersection it is likely that the weekday AM peak hour will be the critical time period with more vehicles attempting the right turn into the University. The T&TPA report did not contain traffic data for the weekday AM peak. In the absence of AM peak data, the PM figures have been reversed to provide representative statistics as shown in **Figure 7**.



**Figure 7: University Ent Int – Existing weekday AM Peak hour traffic volumes**

Based on the above assumptions, a SIDRA Intersection analysis was carried out for the intersection for the existing weekday AM peak. The movement summary output from the SIDRA assessment is shown in **Figure 8**. Full results of the SIDRA analysis are provided in **Appendix B**.

## MOVEMENT SUMMARY

**Site: Uni existing AM**

New Site

Giveway / Yield (Two-Way)

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Leeds Parade											
2	T1	1	5.0	0.001	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
3	R2	82	5.0	0.047	8.0	LOS A	0.2	1.6	0.02	0.67	42.7
Approach		83	5.0	0.047	7.9	NA	0.2	1.6	0.02	0.66	42.8
East: University											
4	L2	27	5.0	0.018	7.5	LOS A	0.1	0.5	0.01	0.63	43.1
6	R2	1	5.0	0.018	7.5	LOS A	0.1	0.5	0.01	0.63	43.1
Approach		28	5.0	0.018	7.5	LOS A	0.1	0.5	0.01	0.63	43.1
North: Leeds Parade											
7	L2	1	5.0	0.001	3.7	LOS A	0.0	0.0	0.00	0.40	46.3
8	T1	1	5.0	0.001	3.7	LOS A	0.0	0.0	0.00	0.40	46.3
Approach		2	5.0	0.001	3.7	NA	0.0	0.0	0.00	0.40	46.3
All Vehicles		113	5.0	0.047	7.7	NA	0.2	1.6	0.02	0.65	43.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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INTERSECTION 6**

**Figure 8: University Ent Int - Existing weekday AM Peak hour traffic movements**

From the table in **Figure 8**, it can be seen that all movements operate at a LOS A, with the worst average delay of 8.0 seconds for the right turn movement off Leeds Parade into the University.

Whilst this is unchanged from the PM peak the overall average delay for all legs increased slightly from the PM peak confirming the AM peak is the critical time period.

# Traffic Generation and Impacts

## 3.1 SURROUNDING DEVELOPMENT

Construction work is currently underway to provide widening to Leeds Parade immediately to the north of Northern Distributor Road to support the new Bunnings development on the western side of the road and a highway service centre on the eastern side of the road. Primary ingress and egress to the Bunnings development will be via Leeds Parade whilst egress only for the highway service centre will be via Leeds Parade (access via Hanrahan Place).

Traffic generated by these two developments will be included in the assessment of the future traffic generation and impacts on the surrounding road network.

## 3.2 TRAFFIC DISTRIBUTION ASSUMPTIONS

As noted elsewhere, the subject development is only one traffic generator in the locality that will affect the movement and distribution of traffic. Therefore, the assumptions determining how traffic would be distributed is set down in the following sections for each traffic generator.

### 3.2.1 PROPOSED SUBDIVISION

For the purposes of further assessment, the following assumptions have been made about traffic distribution from the proposed subdivision development:

For peak periods:

- Leeds Parade (north)
  - AM Peaks: 75% Southbound and 25% Northbound
  - PM Peaks: 75% Northbound and 25% Southbound

Traffic exiting and leaving the subdivision would be expected to distribute from the Leeds Parade/Northern Distributor Road intersection on the following basis:

- 60% of traffic travelling to/from the subdivision would originate from/travel to Leeds Parade south;
- 30% of the traffic would originate from/travel to Northern Distributor Road (west);
- 10% of traffic would originate from/travel to Northern Distributor Road (east); and

5% of traffic entering and leaving the university would originate from/travel to the new subdivision.

### 3.2.2 BUNNINGS

The distribution of traffic generated by the Bunnings development will be as detailed in the T&TPA report included in **Appendix A**.

### 3.2.3 HIGHWAY SERVICE CENTRE

For the purposes of further assessment, the following assumptions have been made about traffic distribution from the new service centre currently under construction:

For additional traffic generated:

- 25% of the traffic would originate from/travel to Northern Distributor Road (west);
- 25% of traffic would originate from/travel to Northern Distributor Road (east);



- 25% of traffic would originate from/travel to Leeds Parade (north); and
- 25% of traffic would originate from/travel to Leeds Parade (south)

For passing traffic (linked trips):

- 25% of the traffic would originate from Northern Distributor Road (west);
- 25% of traffic would originate from Northern Distributor Road (east);
- 25% of traffic would originate from Leeds Parade (north); and
- 25% of traffic would originate from Leeds Parade (south)

### 3.3 TRAFFIC GENERATION

#### 3.3.1 PROPOSED SUBDIVISION

The Roads and Traffic Authority's *Guide to Traffic Generating Developments* publishes data on the traffic generating potential of various development ranging from residential subdivisions, commercial premises, retail premises and industrial developments.

The RTA first published the *Guide to Traffic Generating Developments* in 1991, before its revision in 2001. It is currently being further revised and as interim measure the Roads and Maritime has published updated traffic survey data for a range of development types. Of relevance to this TIA is data provided in relation to low density residential dwellings. The summary figures are provided for the Sydney area and regional areas. A total of five regional sites were surveyed, including one in the Calare area of Orange. The figures for regional areas are relevant to this TIA and are reproduced in **Table 3.1**.

**Table 3.1 – Summary of land use traffic generation – low density residential**

Rates	Regional Average (movements/dwelling)	Regional Range (movements/dwelling)
Daily vehicle trips per dwelling	6.34	5.16-7.12*
Peak- vehicle trips per dwelling	0.79	0.61-0.9

Source: RMS Guide to Traffic Generating Developments – updated traffic surveys August 2013

\* It is noted that the Orange (Calare) figures are the highest in the range at 7.12 but also that car occupancy for Orange was highest at 1.42 persons/vehicle (compared to average regional occupancy of 1.34) and also highest in the peak vehicle trips per dwelling at 0.9

To be conservative, the Orange figures of 7.12/day and 0.9/peak hour have been used for the purposes of this TIA.

Based on the stated traffic generation rates and the assumed average yield, the traffic generated by the development of the proposed subdivision is:

#### Daily Vehicle Trips:

450 lots x 7.12 trips per lot per day = 3,204 vehicle trips per day

#### Peak Hour Trips:

450 lots x 0.9 trips per lot per hour = 405 vehicle trips per hour

#### 3.3.2 BUNNINGS

Figures for the generation of traffic associated with the development of the Bunnings site on the corner of Leeds Parade and Northern Distributor Road will be as detailed in the T&TPA report included in **Appendix A**.

### 3.3.3 HIGHWAY SERVICE CENTRE

Traffic figures for the approved highway service centre have been determined by reference to the 2002 RTA *Guide to Traffic Generating Development*. The Guide identifies that peak hour traffic generation associated with a service station is determined by the formula:

$0.04 A(S) + 0.3 A(F)$ , where

$A(S)$  = area of the site ( $m^2$ ) and

$A(F)$  = gross floor area of the convenience store

The area of the site is understood to be approximately  $5,700m^2$  and the area of the approved convenience store is  $200m^2$ .

Therefore:

Peak Hour Trips (Service Station):

$$0.04 * 5700 + 0.3 * 200 = 288 \text{ vehicle trips per hour}$$

The development also contains a drive in take away food outlet. The Guide recommends adopting a figure of 100 vehicles for peak hour traffic generation for a KFC or equivalent. It is noted that MacDonalds has higher recommended rates however given the proximity of the North Orange MacDonalds, the likelihood that a MacDonalds would be developed here is low.

Therefore, the final traffic generation figure for the site becomes:

Peak Hour Trips (Service Centre Total):

$$288 + 100 = 388 \text{ vehicle trips per hour}$$

The Guide notes the proportion of passing trade is typically 50%. Hence 194 vehicle trips per hour are new trips with the remaining 194 vehicle trips per hour coming from the existing traffic stream.

## 3.4 TRAFFIC IMPACTS

### 3.4.1 BASIS OF ASSESSMENT

The impact of the traffic generated by the development of the proposed subdivision on Leeds Parade and Northern Distributor Road will be assessed in terms of:

- i. Traffic volume; and
- ii. Intersection capacity.

The impact of the increased traffic on Leeds Parade and Northern Distributor Road will be assessed. The operational capacities of the roundabout at the intersection of Leeds Parade and Northern Distributor Road and the intersection on Leeds Parade at the University entrance will also be analysed and evaluated.

### 3.4.2 TRAFFIC VOLUME

The expected peak hour traffic volume generated from the subdivision will impact on the existing peak hour traffic volume on Leeds Parade and Northern Distributor Road. The increase in peak hour traffic volume on these two roads is summarised in **Table 3.2**.

**Table 3.2 – Comparison of Existing & Post Development Weekday PM Peak Hour Traffic Volumes**

Road Location	Lane	Weekday PM Peak Hour Traffic (vehicles/hour)		Increase
		Existing	Post Development	
Northern Distributor Road (east of Leeds Parade)	Eastbound	241	342	42%
	Westbound	321	459	43%
<b>TOTAL</b>		<b>562</b>	<b>801</b>	<b>43%</b>
Northern Distributor Road (west of Leeds Parade)	Eastbound	362	556	54%
	Westbound	580	768	32%
<b>TOTAL</b>		<b>942</b>	<b>1324</b>	<b>41%</b>
Leeds Parade (north of Northern Distributor Road)	Northbound	28	356	1,171%
	Southbound	83	208	151%
<b>TOTAL</b>		<b>111</b>	<b>564</b>	<b>408%</b>
Leeds Parade (south of Northern Distributor Road)	Northbound	322	652	102%
	Southbound	242	405	67%
<b>TOTAL</b>		<b>564</b>	<b>1057</b>	<b>87%</b>

As expected, the greatest percentage increase in the weekday PM peak hour traffic following the full development of the proposed subdivision occurs on the northbound lane of Leeds Parade (north) with an increase in traffic volume of approximately 1,171%.

Whilst the percentage increase in traffic volume on Leeds Parade (north) appears extremely excessive it should be noted that this increase is from a very low existing traffic volume and the final post development traffic volume includes the traffic generated from the proposed Service Centre. Furthermore reference to **Section 2.4** of this report indicated that Leeds Parade has an existing capacity of 900 vehicles per hour per lane that is significantly more than the predicted post development peak traffic of 356 vehicles per hour.

**Table 3.3** lists the peak hour traffic volume of the surrounding roads when compared against the existing capacity of the road.

**Table 3.3 – Comparison of Post Development Weekday PM Peak Hour Traffic Volumes to Capacity**

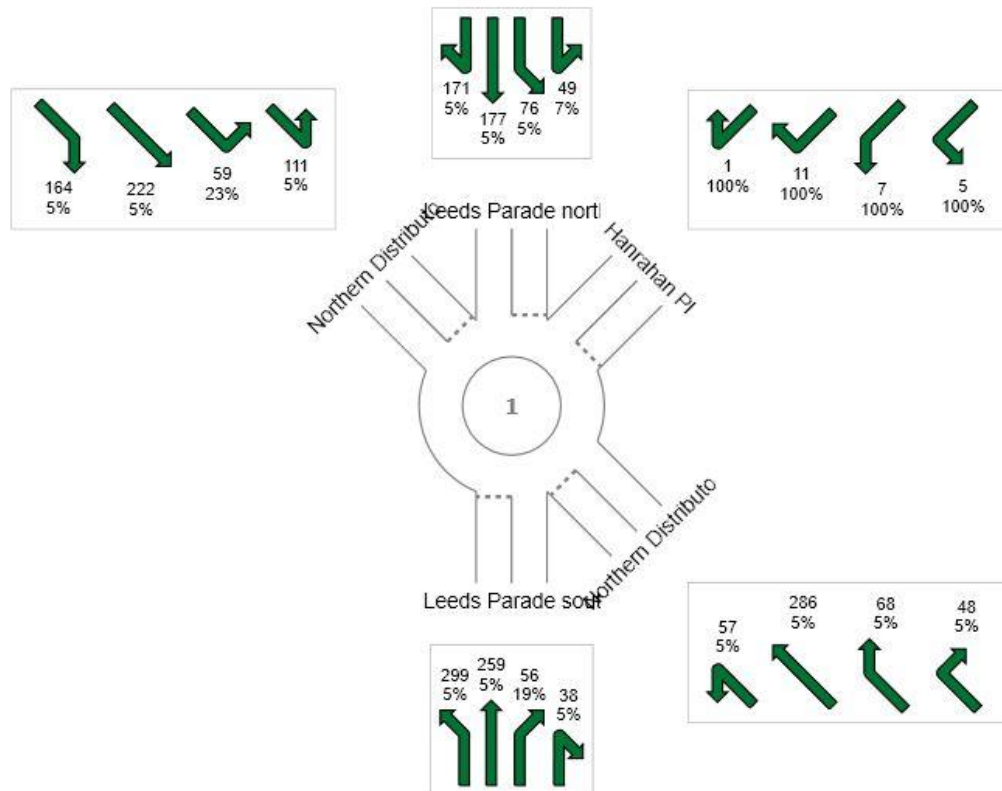
Road Location	Lane	Weekday PM Peak Hour Traffic (vehicles/hour)		Percentage of Capacity
		Post Development	Capacity	
Northern Distributor Road (east of Leeds Parade)	Eastbound	342	1,200	29%
	Westbound	459	1,200	38%
Northern Distributor Road (west of Leeds Parade)	Eastbound	556	1,200	46%
	Westbound	768	1,200	64%
Leeds Parade (north of Northern Distributor Road)	Northbound	356	900	40%
	Southbound	208	900	23%
Leeds Parade (south of Northern Distributor Road)	Northbound	652	900	72%
	Southbound	405	900	45%

It can be seen from **Table 3.3** that all roads are well below their capacity with the northbound lane of Leeds Parade (south) closest to capacity at 72%. Hence all roads are able to accommodate the increased additional peak hour traffic generated by the proposed development.

### 3.4.3 INTERSECTION ANALYSIS

#### 3.4.3.1 Leeds Parade and Northern Distributor Road

Based on the assumptions outlined in **Section 2.6.2**, a SIDRA analysis was carried out for the Leeds Parade and Northern Distributor Road intersection under post development conditions during the weekday PM peak hour. The post development traffic and turning movements used in the SIDRA analysis is shown in **Figure 9**.



**Figure 9: NDR/Leeds Prd Int – Post Development weekday PM Peak hour traffic volumes**

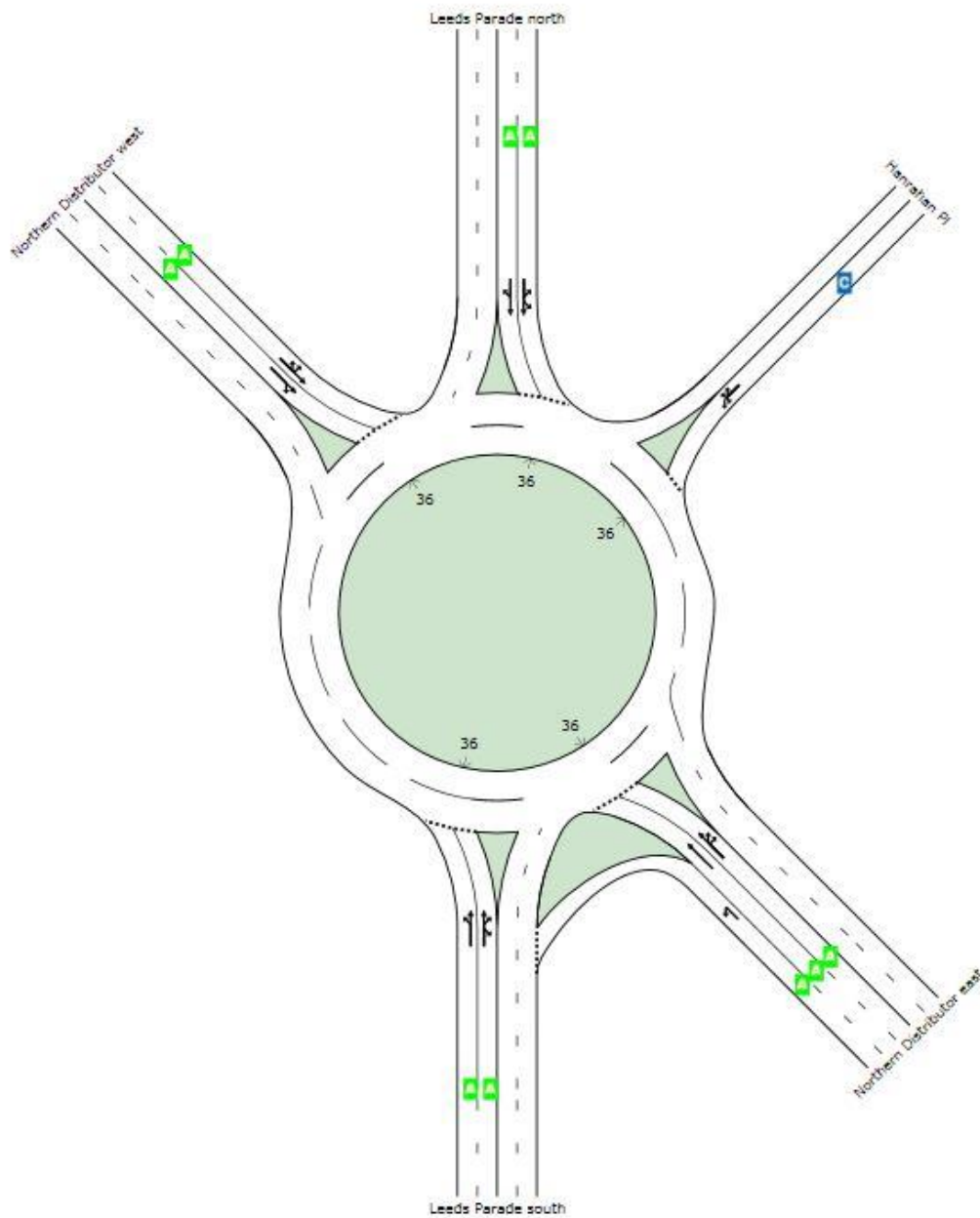
**Figure 10** illustrates the post development weekday PM peak hour Level of Service (LOS) for the Leeds Parade/Northern Distributor Road intersection and **Figure 11** provides the SIDRA movement summary. Full results of the SIDRA analysis are provided in **Appendix B**.



## LEVEL OF SERVICE

**Site: WD PM (NDR/Leeds) - proposed +bunnings + service centre**

New Site  
Roundabout



	South	Southeast	Northeast	North	Northwest	Intersection
LOS	A	A	C	A	A	A

**Figure 10: NDR/Leeds Prd Int - Post Development weekday PM Peak LOS**

## MOVEMENT SUMMARY

Site: WD PM (NDR/Leeds) - proposed +bunnings + service centre

New Site  
Roundabout

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Leeds Parade south											
1a	L1	299	5.0	0.330	6.6	LOS A	1.7	12.4	0.58	1.08	49.6
2	T1	259	5.0	0.330	9.5	LOS A	1.7	12.4	0.60	1.35	48.1
3a	R1	56	19.0	0.330	10.4	LOS A	1.6	12.1	0.60	1.43	47.7
3b	R3	38	5.0	0.330	10.4	LOS A	1.6	12.1	0.60	1.43	47.7
Approach		652	6.2	0.330	8.3	LOS A	1.7	12.4	0.59	0.62	48.7
SouthEast: Northern Distributor east											
21b	L3	57	5.0	0.037	8.0	LOS A	0.2	1.2	0.36	1.10	49.3
22	T1	286	5.0	0.163	7.0	LOS A	0.8	6.1	0.51	1.05	49.4
23a	R1	68	5.0	0.163	11.5	LOS A	0.8	5.7	0.53	1.43	46.7
23	R2	48	5.0	0.163	11.5	LOS A	0.8	5.7	0.53	1.43	46.7
Approach		459	5.0	0.163	8.3	LOS A	0.8	6.1	0.49	0.57	48.6
NorthEast: Hanrahan Pl											
24	L2	5	100.0	0.057	32.2	LOS C	0.2	2.7	0.62	1.50	42.2
24a	L1	7	100.0	0.057	32.2	LOS C	0.2	2.7	0.62	1.50	42.2
26	R2	11	100.0	0.057	32.2	LOS C	0.2	2.7	0.62	1.50	42.2
26b	R3	1	100.0	0.057	32.2	LOS C	0.2	2.7	0.62	1.50	42.2
Approach		24	100.0	0.057	32.2	LOS C	0.2	2.7	0.62	0.75	42.2
North: Leeds Parade north											
7b	L3	49	7.0	0.240	7.1	LOS A	1.2	8.5	0.56	1.13	49.4
7a	L1	76	5.0	0.240	7.1	LOS A	1.2	8.5	0.56	1.13	49.4
8	T1	177	5.0	0.240	8.9	LOS A	1.2	8.5	0.56	1.25	48.1
9b	R3	171	5.0	0.240	14.5	LOS A	1.1	8.2	0.58	1.60	45.0
Approach		473	5.2	0.240	10.4	LOS A	1.2	8.5	0.57	0.67	47.2
NorthWest: Northern Distributor west											
27b	L3	111	5.0	0.271	8.6	LOS A	1.5	11.0	0.56	1.24	48.8
27	L2	59	23.0	0.271	8.6	LOS A	1.5	11.0	0.56	1.24	48.8
28	T1	222	5.0	0.271	9.7	LOS A	1.5	11.0	0.56	1.33	47.9
29a	R1	164	5.0	0.271	11.3	LOS A	1.4	10.3	0.57	1.46	46.9
Approach		556	6.9	0.271	9.8	LOS A	1.5	11.0	0.56	0.67	47.9
All Vehicles		2164	7.0	0.330	9.4	LOS A	1.7	12.4	0.56	0.64	47.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akpelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

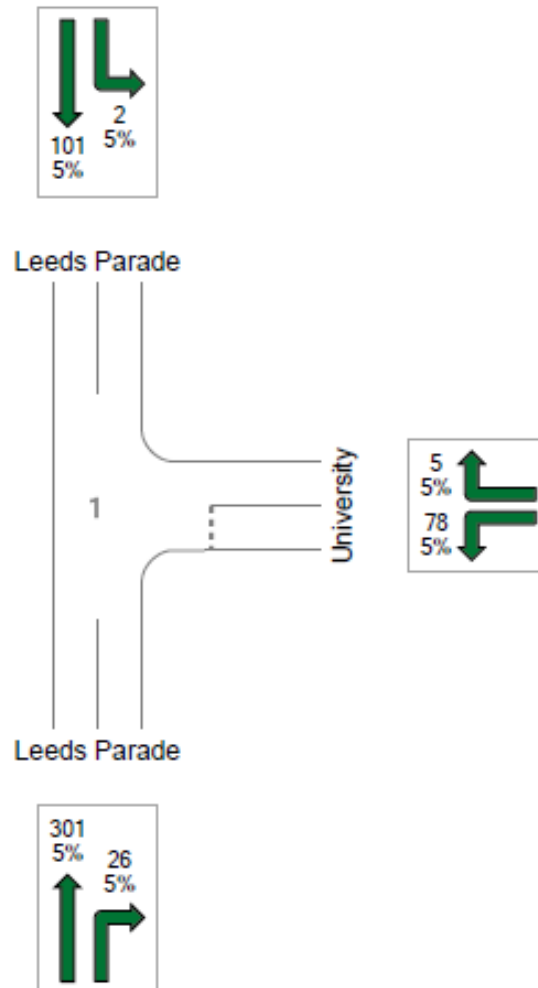
**Figure 11: NDR/Leeds Prd Int – Post Development weekday PM Peak hour traffic movements**

From the table in **Figure 11**, it can be seen that the worst movement remains as the movement for vehicles existing Hanrahan Place, with a LOS C and a slightly lower average delay of 32.2 seconds.

The increase in traffic from the proposed development provides for a more even distribution of traffic entering and leaving the roundabout that in turn results in an increase in the LOS on some legs with no decrease in the LOS on any legs. The overall average delay at the roundabout decreases from 14.8 seconds under existing conditions to 9.5 seconds under post development conditions.

### 3.4.3.2 Leeds Parade and University Access

Based on the assumptions outlined in **Section 2.6.3**, a SIDRA analysis was carried out for the Leeds Parade/University Entrance intersection under post development conditions during the weekday PM peak hour. The post development traffic and turning movements used in the SIDRA analysis is shown in **Figure 12**.



**Figure 12: University Ent Int. – Post Development weekday PM Peak hour traffic volumes**

**Figure 13** illustrates the post development weekday PM peak hour Level of Service (LOS) for the Leeds Parade/University Entrance intersection and **Figure 14** provides the SIDRA movement summary. Full results of the SIDRA analysis are provided in **Appendix B**.

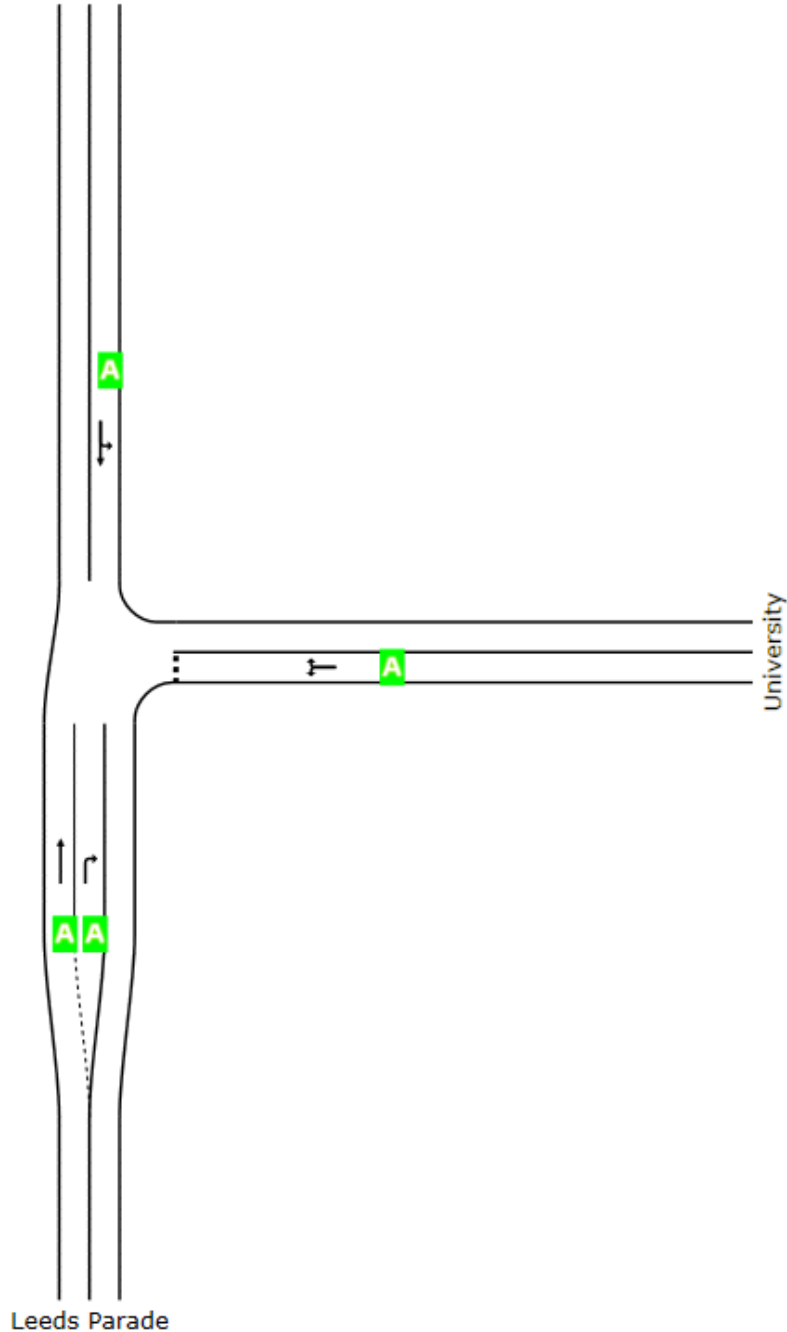
## LEVEL OF SERVICE

**Site: Uni post PM**

New Site

Giveway / Yield (Two-Way)

Leeds Parade



Leeds Parade

	South	East	North	Intersection
LOS	NA	A	NA	NA

**Figure 13: University Ent Int – Post Development weekday PM Peak LOS**

## MOVEMENT SUMMARY

Site: Uni post PM

New Site  
Giveaway / Yield (Two-Way)

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Leeds Parade											
2	T1	301	5.0	0.159	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
3	R2	26	5.0	0.016	8.3	LOS A	0.1	0.5	0.21	0.61	42.2
Approach		327	5.0	0.159	0.7	NA	0.1	0.5	0.02	0.05	49.2
East: University											
4	L2	78	5.0	0.061	8.0	LOS A	0.2	1.8	0.20	0.60	42.5
6	R2	5	5.0	0.061	8.0	LOS A	0.2	1.8	0.20	0.60	42.5
Approach		83	5.0	0.061	8.0	LOS A	0.2	1.8	0.20	0.60	42.5
North: Leeds Parade											
7	L2	2	5.0	0.055	0.2	LOS A	0.0	0.0	0.00	0.02	49.8
8	T1	101	5.0	0.055	0.2	LOS A	0.0	0.0	0.00	0.02	49.8
Approach		103	5.0	0.055	0.2	NA	0.0	0.0	0.00	0.02	49.8
All Vehicles		513	5.0	0.159	1.8	NA	0.2	1.8	0.04	0.13	48.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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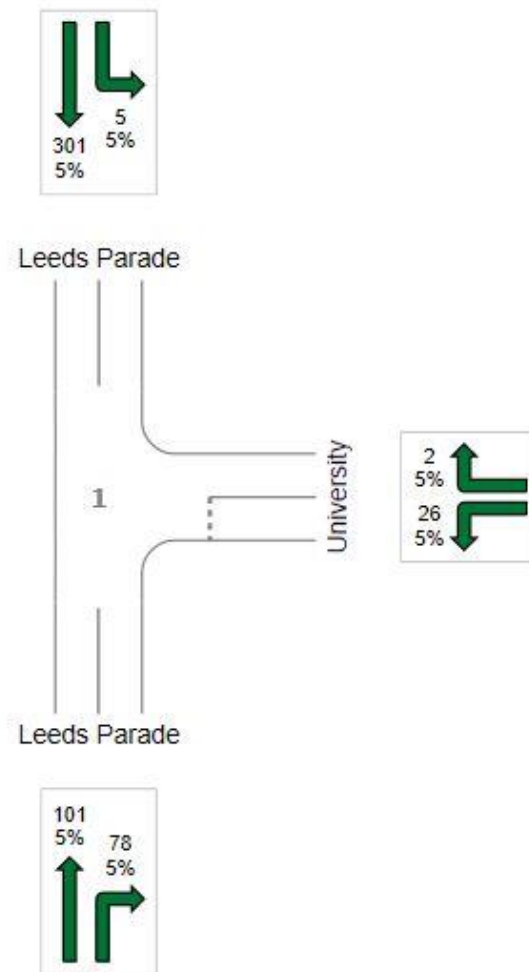
**SIDRA  
INTERSECTION 6**

**Figure 14: University Ent Int – Post Development weekday PM Peak hour traffic movements**

From the table in **Figure 14**, it can be seen that all movements still operate at a LOS A, with the worst average delay of 8.3 seconds for the right turn movement off Leeds Parade into the University. The 95% back of vehicles is less than 1 vehicle indicating the existing right turn lane length is adequate.

As stated in **Section 2.6.3** it is expected that the weekday AM peak hour movements will be the critical time period with more vehicles attempting the right turn into the University. Based on the assumptions outlined in **Section 2.6.3**, a SIDRA analysis was carried out for the Leeds Parade/University Entrance intersection under post development conditions during the weekday AM peak hour. The post development traffic and turning movements used in the SIDRA analysis is shown in **Figure 15**.





**Figure 15: University Ent Int. – Post development weekday AM Peak hour traffic volumes**

**Figure 16** illustrates the post development weekday AM peak hour Level of Service (LOS) for the Leeds Parade/University Entrance intersection and **Figure 17** provides the SIDRA movement summary. Full results of the SIDRA analysis are provided in **Appendix B**.

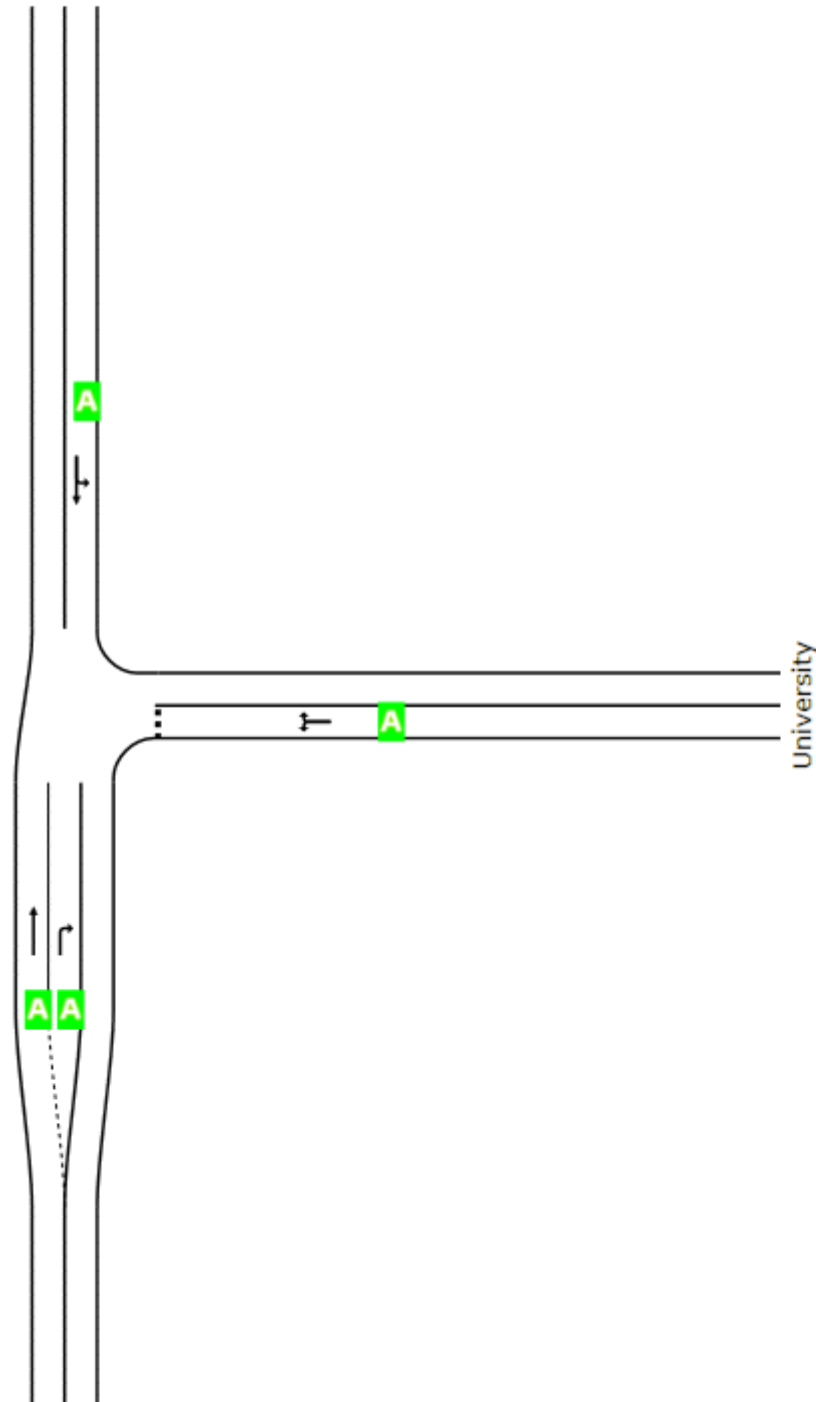
## LEVEL OF SERVICE

Site: Uni post AM

New Site

Giveway / Yield (Two-Way)

Leeds Parade



Leeds Parade

	South	East	North	Intersection
LOS	NA	A	NA	NA

Figure 16: University Ent Int – Post Development weekday AM Peak LOS

## MOVEMENT SUMMARY

Site: Uni post AM

New Site

Giveway / Yield (Two-Way)

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Flows Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Leeds Parade											
2	T1	101	5.0	0.053	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
3	R2	78	5.0	0.060	9.1	LOS A	0.3	1.9	0.40	0.66	41.6
Approach		179	5.0	0.060	4.0	NA	0.3	1.9	0.17	0.29	45.9
East: University											
4	L2	26	5.0	0.025	8.7	LOS A	0.1	0.7	0.38	0.63	41.8
6	R2	2	5.0	0.025	8.7	LOS A	0.1	0.7	0.38	0.63	41.8
Approach		28	5.0	0.025	8.7	LOS A	0.1	0.7	0.38	0.63	41.8
North: Leeds Parade											
7	L2	5	5.0	0.162	0.1	LOS A	0.0	0.0	0.00	0.02	49.8
8	T1	301	5.0	0.162	0.1	LOS A	0.0	0.0	0.00	0.02	49.8
Approach		306	5.0	0.162	0.1	NA	0.0	0.0	0.00	0.02	49.8
All Vehicles		513	5.0	0.162	1.9	NA	0.3	1.9	0.08	0.15	47.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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**SIDRA  
INTERSECTION 6**

**Figure 17: University Ent Int – Post Development weekday AM Peak hour traffic movements**

From the table in **Figure 17**, it can be seen that all movements still operate at a LOS A, with the worst average delay of 9.1 seconds for the right turn movement off Leeds Parade into the University. The 95% back of vehicles is less than 1 vehicle indicating the existing right turn lane length is adequate.

### **3.4.4 IMPACT SUMMARY**

The assessment of the impact of the traffic generated by the development of the proposed subdivision on the surrounding road network has determined that the impact on:

- i. Traffic Volume; and
- ii. Intersection Capacity

The greatest increase in the weekday PM peak hour traffic following the development of the proposed subdivision occurred on the northbound lane of Leeds Parade (south) with an increase in traffic volume of approximately 1,171%.

Whilst the percentage increase in traffic volume on the northbound lane of Leeds Parade (north) appears extremely excessive, the total peak traffic volume of 356 vehicles per hour is significantly less than the existing capacity of 900 vehicles per hour per lane.

All roads assessed are well below their capacity with the northbound lane of Leeds Parade (south) closest to capacity at 72%. Hence all roads are able to accommodate the increased additional peak hour traffic generated by the proposed development.

The increase in traffic from the proposed development provides for a more even distribution of traffic entering and leaving the roundabout at the intersection of Leeds Parade and Northern Distributor Road. SIDRA analysis of the roundabout showed that this resulted in an increase in the LOS on some legs with no decrease in the LOS on any legs. The overall average delay at the roundabout decreased from 14.8 seconds under existing conditions to 9.5 seconds under post development conditions.

SIDRA analysis of the intersection on Leeds Parade at the University entrance showed that that all movements at the intersection operate at a LOS A under post development conditions, with the worst average delay of 9.1 seconds for the right turn movement off Leeds Parade into the University during the weekday AM peak hour. The 95% back of vehicles for this movement is less than 1 vehicle indicating the existing right turn lane length is adequate.

Whilst the expected increases in traffic volume are significant, the capacity of the existing surrounding roads and intersections analysed are sufficient to cater for the post development traffic volumes and to disperse such traffic into the surrounding road network.

# Recommendations

This Traffic Impact Assessment has evaluated the traffic generating potential of a proposed 450 lot rural residential subdivision in North Orange and has assessed the impact of the additional traffic generated on the surrounding roads and intersections.

The TIA has found that whilst the expected increases in traffic volume are significant, the capacity of the existing surrounding roads and intersections analysed are sufficient to cater for the post development traffic volumes and to disperse such traffic into the surrounding road network.

In completing the assessment of the impact of the traffic generated by the proposed development, the following recommendations are made:

- The design of all internal roads within the proposed subdivision shall be carried out to appropriate standards and the requirements of Orange City Council;
- The design of the subdivision should include a threshold entry treatment on Leeds Parade to clearly indicate the beginning of the large lot residential subdivision;
- The existing bicycle/pedestrian path on Leeds Parade should be extended to and into the proposed subdivision;
- Consideration should be given to widening Leeds Parade to provide full line marking and sealed shoulders to improve road safety and minimise future maintenance costs.



# References

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AUSTROADS (2009) Guide to Road Design Part 4A. Unsignalised and Signalised Intersections

---

AUSTROADS (2013) Guide to Traffic Management Part 12. Traffic Impacts of Developments

---

AUSTROADS (2013) Guide to Traffic Management Part 3. Traffic Studies and Analysis.

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Ogden, K.W. and Bennett, D.W. (Eds) 1984) Traffic Engineering Practice. Third Edition. Dept of Civil Engineering Monash University.

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Roads and Traffic Authority (2002) Guide to Traffic Generating Developments.

---

Transport and Traffic Planning Associates (2013) Proposed Bunnings Warehouse - Assessment of Traffic and Parking Implications

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# **Appendix A**

---

## **T&TPA TRAFFIC REPORT**

**PROPOSED  
BUNNINGS WAREHOUSE  
CNR NORTHERN DISTRIBUTOR ROAD  
AND LEEDS PARADE, ORANGE**

***Assessment of Traffic and  
Parking Implications***

May 2013

Reference 12084

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## LIST OF ILLUSTRATIONS

FIGURE 1	LOCATION
FIGURE 2	SITE
FIGURE 3	ROAD NETWORK
FIGURE 4	TRAFFIC CONTROLS

# 1. INTRODUCTION

---

This report has been prepared to accompany a Development Application to Orange City Council for a proposed new Bunnings Warehouse on a site with frontages to Northern Distributor Road and Leeds Parade at Orange (Figure 1).

The recent completion of the Northern Distributor Road providing a bypass to the Orange Town Centre has enabled upgraded vehicle access for urban development in the area to the north of the Town Centre.

The Bunnings development site is grassed farmland and the proposed scheme will enable replacement of the small existing Bunnings store which is located to the southeast of the Town Centre.

The proposed development will be a contemporary Bunnings warehouse comprising:

Warehouse	9,490 m <sup>2</sup>
Trade Area	4,780 m <sup>2</sup>
Nursery and bagged goods	3,020 m <sup>2</sup>
Total Retail Area	17,290 m <sup>2</sup>
Carparking	330 spaces

The purpose of this report is to:

- \* describe the site and the proposed development scheme
- \* describe the existing road network and traffic conditions
- \* assess the adequacy of the proposed parking provision
- \* assess the traffic potential implications of the development
- \* assess the proposed access, internal circulation and servicing arrangements.



**FIG 1**

## 2. PROPOSED DEVELOPMENT SCHEME

---

### 2.1 SITE, CONTEXT AND EXISTING USE

The development site (Figure 2) is part of a consolidation of lots being an irregular shaped area of some 44,798m<sup>2</sup> within a total lot area of 8.58 ha. The site has a frontage to the northern side of Northern Distributor Road and to the western side of Leeds Parade and is located some 2.5 kms to the north of the City Centre.

The site, which is grassed farm land with a residence and out buildings, is bounded to the west by the Main Western Railway Line.

The surrounding uses comprise:

- \* the industrial site just to the west and bus depot just to the east
- \* the Orange Marketplace Centre further to the west
- \* the new residential development to the west extending along both sides of NDR
- \* the farmland extending to the north
- \* the Orange Grove Homemakers Centre to the southeast

### 2.2 PROPOSED DEVELOPMENT

The development scheme involves some cut and fill of the site to provide level platforms for building and hardstand areas. The new Bunnings warehouse building will occupy the central and northern part of the site with the nursery located on the western side and carparking along the southern frontage area.

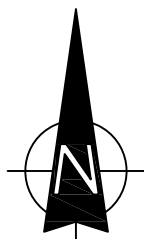
The proposed development scheme will comprise a single warehouse level with at grade carparking as follows:

Warehouse	-	9,490m <sup>2</sup>
Trade area	-	4,780m <sup>2</sup>
Nursery and bagged goods	-	3,020m <sup>2</sup>
<b>Total Retail Area:</b>		<b>17,290m<sup>2</sup></b>





**LEGEND**



**SITE**

**FIG 2**

A total of 330 parking spaces will be provided with vehicle access comprising:

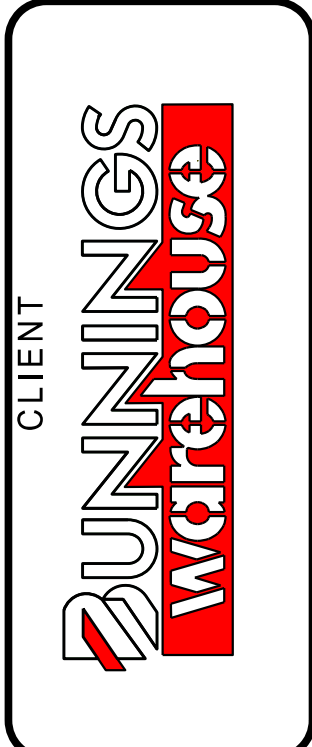
- \* ingress and egress for the carpark on Northern Distributor (left turn IN/OUT only)
- \* ingress and egress for the carpark on Leeds Parade towards the centre of the site
- \* ingress and egress for service vehicles on Leeds Parade in the northern part of the frontage

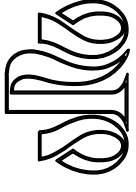
Details of the proposed development scheme are provided on the plans prepared by John R Brogan and Associates which accompany the Development Application and are reproduced in part overleaf.



NO.	AMENDMENT	DATE	NO.	AMENDMENT	DATE
1	PRELIMINARY	30.01.13			
2	PRELIMINARY	13.05.13			
3	PRELIMINARY	13.05.13			

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**PROPOSED BUNNINGS WAREHOUSE ORANGE**  
NORTHERN DISTRIBUTOR RD & LEEDS PDE  
ORANGE NSW 2800

**SITE PLAN - INCL CORNER LOT**  
CONCEPT C

**SCALE :**  
1 : 750

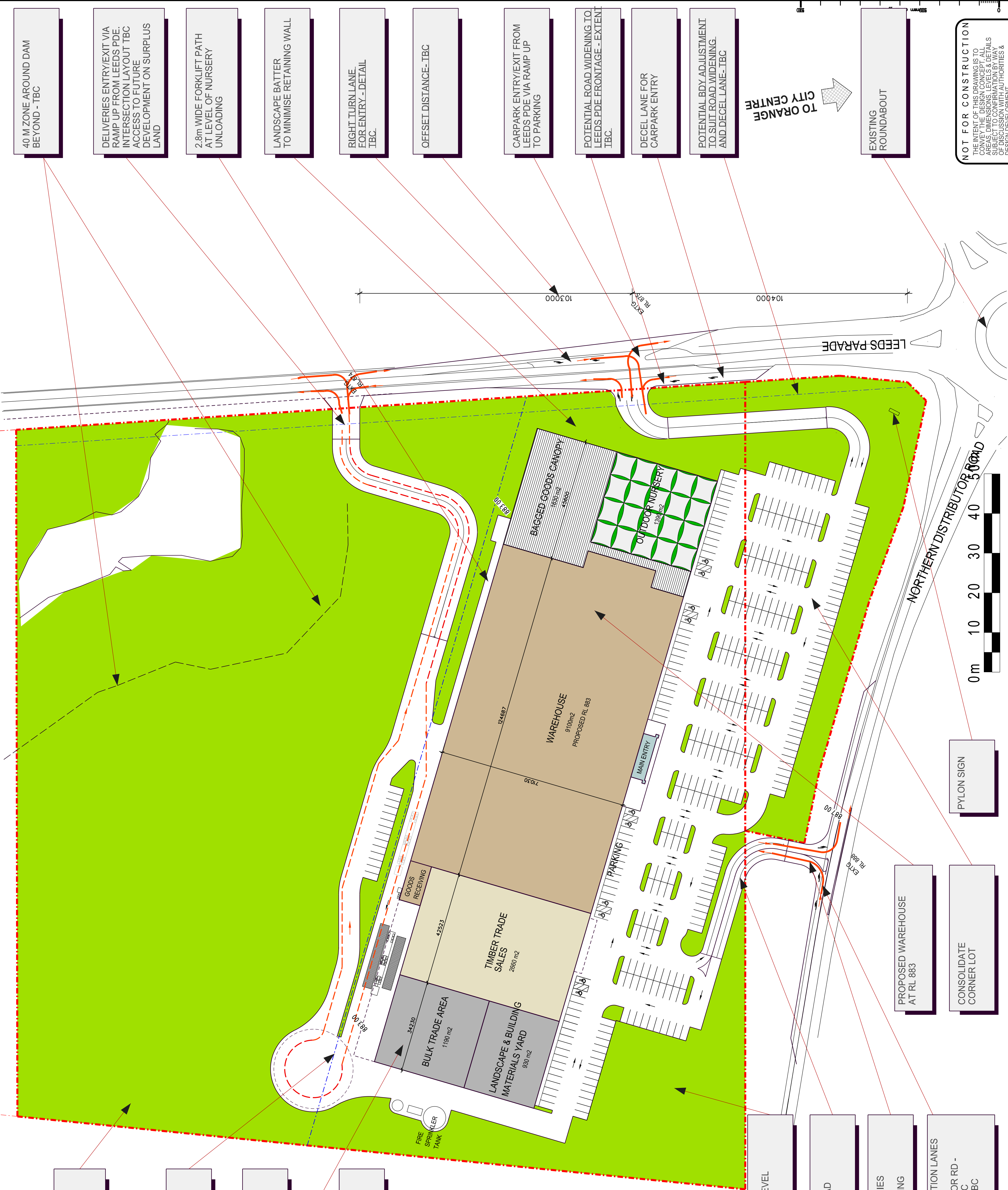
**DATE :**  
JUL 12

**PROJ. NO. :**  
1311

**DRG. NO. :**  
K031

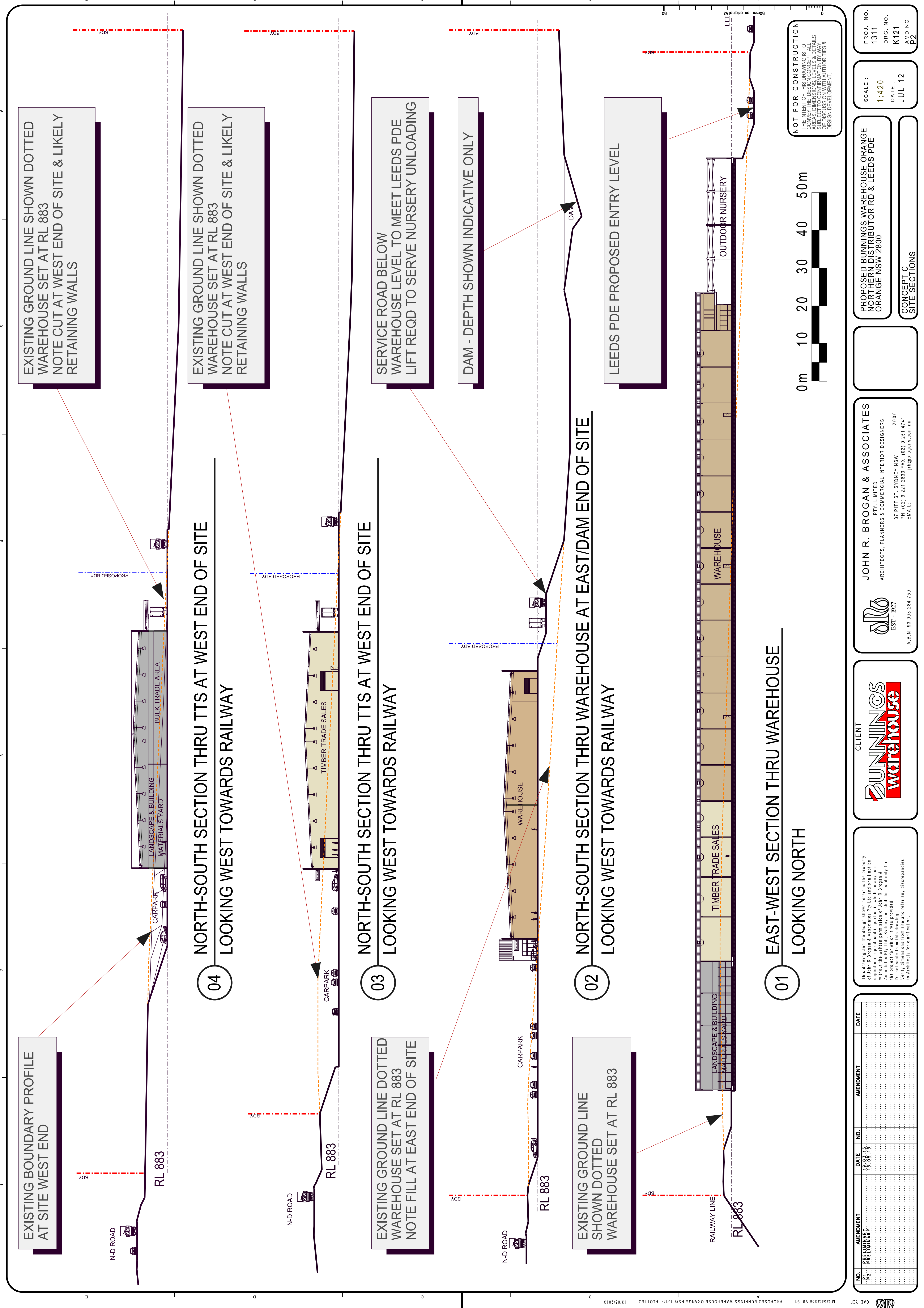
**APP. NO. :**  
P3

AREA ANALYSIS PROPOSED BUNNINGS ORANGE	
WAREHOUSE	9 100 m2
Warehouse	300 m2
Mezzanine	90 m2
Main Entry	9 490 m2
TOTAL WAREHOUSE AREA	2660 m2
Timber Trade Sales	930 m2
Landscape & Building Materials Yard	1190 m2
Bulk Trade Area	4780 m2
TOTAL TRADE AREA	1390 m2
Outdoor Nursery	1630 m2
Bagged Goods Canopy	3 020 m2
TOTAL NURSERY AREA	17 290 m2
TOTAL BUNNINGS AREA	330 CARS
Bunnings Parking Provided	1/52 m2
Carparking Ratio	
Land Size - APPROX. FIGURES TBC (m2)	
MAIN LOT - SOUTH	37 750 m2
CONSOLIDATE CORNER LOT	8 050m2
Proposed BUNNINGS Lot	45 800 m2
SURPLUS LOT - NORTH (INCL DAM)	38 000 m2
TOTAL LAND	85 800m2



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0m 10 20 30 40 50m

NO. 01

PRELIMINARY

AMENDMENT

DATE 19.03.13

NO. 1311

DATE 19.03.13

PROJ. NO. 1311

SCALE : 1:420

DRG. NO. K121

DATE : JUL 12

AMD NO. P2

PROPOSED BUNNINGS WAREHOUSE ORANGE

NORTHERN DISTRIBUTOR RD & LEEDS PDE

ORANGE NSW 2800

CONCEPT C

SITE SECTIONS

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warehouse

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CAD REF : Microstation V8i S1 13/05/2013

PROPOSED BUNNINGS WAREHOUSE ORANGE NS 1311 - PLOTTED

13/05/2013

### 3. ROAD NETWORK AND TRAFFIC CONTROLS

---

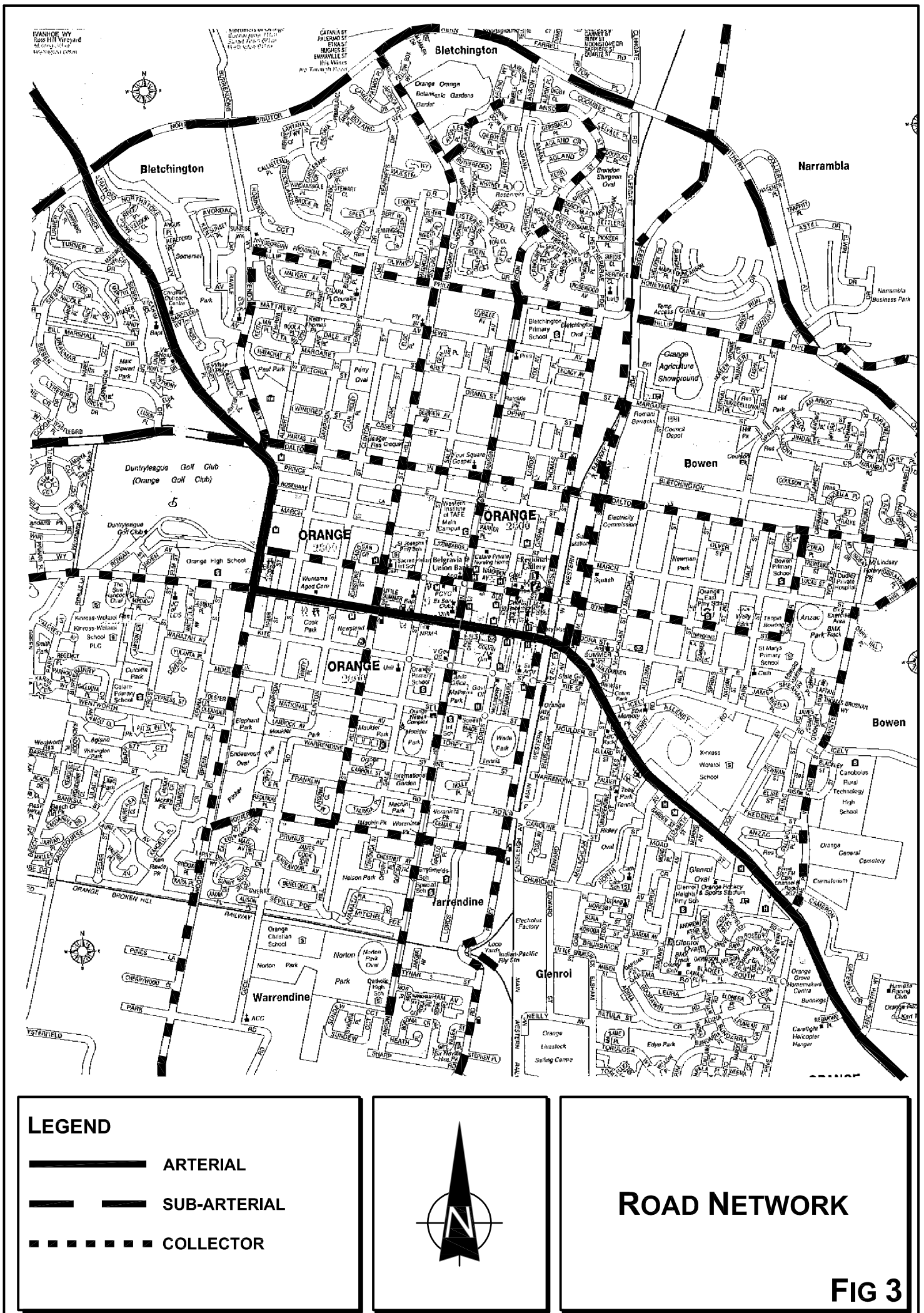
#### 3.1 ROAD NETWORK

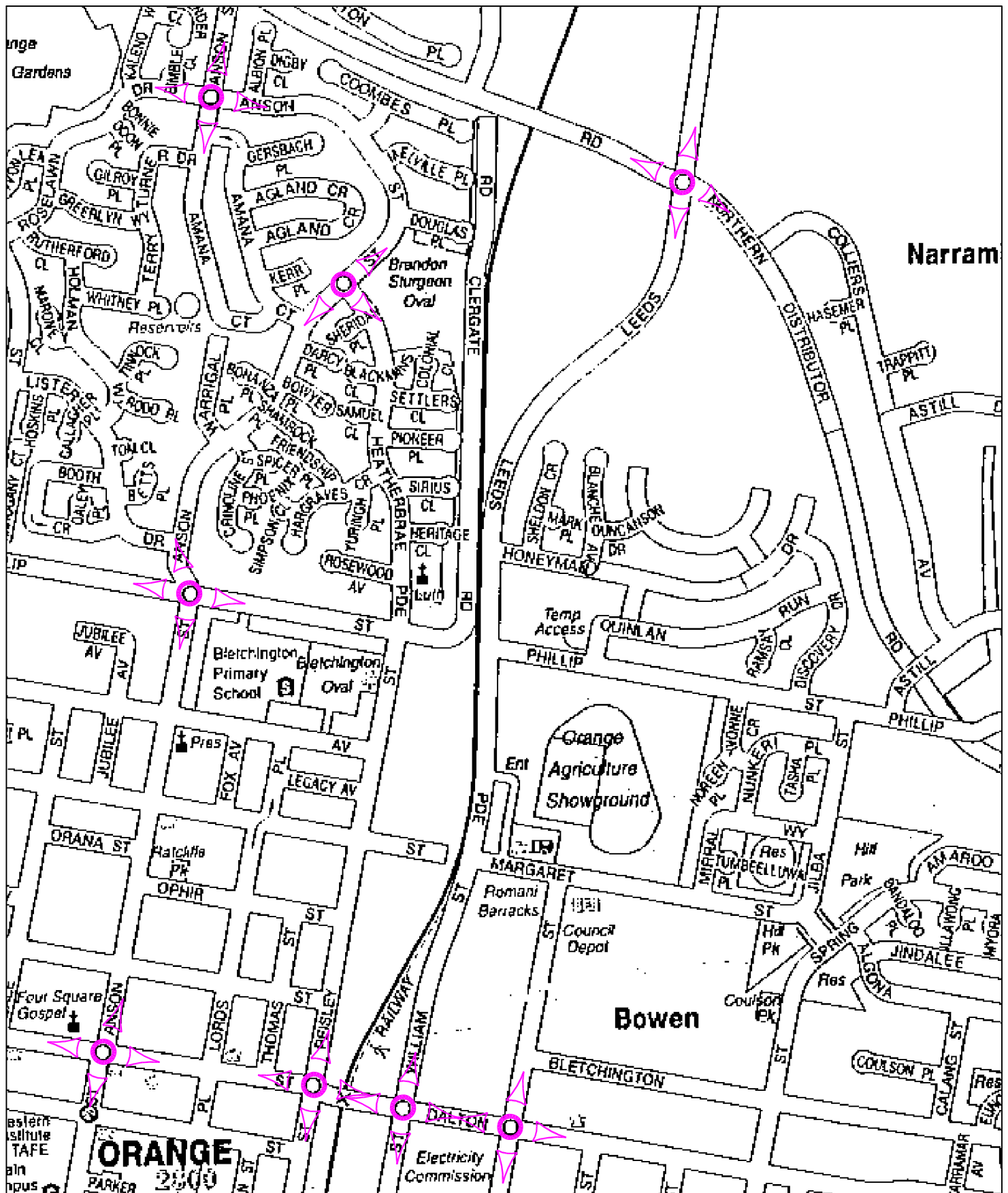
- \* *Mitchell Highway* – a principal State Highway route extending northwest from Bathurst through Orange
- \* *Northern Distributor Road* – a new major collector road linking between the Mitchell Highway and providing a bypass around the Orange City Centre
- \* *William Street/Leeds Parade* – a collector road route connecting northwards from the Orange Centre
- \* *Burrendong Way, Escourt Way, Cargo Road and Forest Road* – major collector routes radiating from Orange Centre
- \* *Hill Street and Phillip Street etc* – collector roads serving the north Orange area

#### 3.2 TRAFFIC CONTROLS

The few traffic controls which have been applied to the road system (Figure 4) in the vicinity of the site comprise:

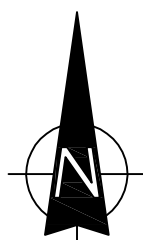
- \* the large roundabout at the Northern Distributor Road and Leeds Parade intersection
- \* the large roundabout at the Mitchell Highway and Northern Distributor Road intersection
- \* the 70 kmph speed limit on NDR and 50 kmph the local and collector roads
- \* the marked footcrossing on Hibberson Street west of Gribble Street
- \* the approved B Double Route along Mitchell Highway and NDR





# LEGEND

- TRAFFIC SIGNAL CONTROL
- ◇ ROUNDABOUT
- RESTRICTED TURNING MOVEMENT



# TRAFFIC CONTROLS

FIG 4

### 3.3 TRAFFIC CONDITIONS

An indication of the traffic conditions on the road system in the vicinity of the site is contained in the results of traffic surveys undertaken by Council which are provided in Appendix A and surveys undertaken during the weekday afternoon and weekend midday periods. The Council data is summarised in the following for total (2-way) traffic follows:

#### North Distributor Road

AWDT	8,400 vph
AM peak	670 vph
PM peak	920 vph
SAT MD	640 vph

#### Leeds Parade (south of NDR)

AWT	3,500 vpd
AM peak	360 vph
PM peak	380 vph
SATMD	280 vph

The results of the traffic surveys undertaken at the intersection are provided in Appendix B and summarised in the following:

		WDPM	WEMD
Northern Distributor Road	EB	195	137
	RT	143	145
	LT	13	3
	WB	267	113
	RT	2	1
	LT	52	11

Leeds Parade	NB	13	9
	RT	30	16
	LT	271	190
	SB	40	10
	RT	31	2
	LT	11	2

There is also an access road for a bus depot which connects into the roundabout however the traffic movements on this access are only minor.

Traffic conditions on the road network in the area during the peak periods, including the weekend, are quite free flowing and satisfactory with no congestion or delay. The site benefits, in traffic terms, from:

- \* the roundabout controlled access available through the Northern Distributor Road and Leeds Parade intersection
- \* the absence of any major traffic generator in the near vicinity



## 4. ACCESS

---

The proposed vehicle access arrangements comprise:

- \* separate adjacent ingress and egress connections for the carpark located on the Northern Distributor Road frontage. There will be a central median island provided in Northern Distributor Road to restrict access movements to left turn IN/OUT with a deceleration lane for the ingress movement
- \* a combined ingress/egress connection for the carpark located on the Leeds Parade frontage with left turn deceleration lane
- \* a combined ingress/egress connection for service/delivery vehicles located on the Leeds Parade frontage to the north of the building

Minor road works will be undertaken to enable the provision of these proposed accesses and they will be located where there will be good sight distances available and sufficiently away from the intersection.

## 5. TRAFFIC

---

Bunnings stores have somewhat unique traffic generation characteristics (compared to other retail type uses) and the circumstances in Country Regional stores are also quite different to those in Capital City Metropolitan Area stores. In order to establish the country regional characteristics TTPA undertook surveys and research at existing Bunnings stores at Tamworth, Bathurst and Coffs Harbour.

The results of this survey data in relation to the peak traffic generation circumstances are and summarised in the following:

	Weekday			Saturday		
	4-5pm			Midday		
	IN	OUT	Total	IN	OUT	Total
Tamworth	124	165	289	186	201	387
Bathurst	74	88	162	111	107	218
Coffs Harbour	108	104	212*	202	180	382*

*\* The existing Coffs Harbour store has a total GFA of 8,523m<sup>2</sup> indicating*

*PM peak – 2.5 vtph/100 m<sup>2</sup>*

*WE peak – 4.5 vtph/100 m<sup>2</sup>*

RMS are in the process of updating their Guide for Traffic Generating Development and engaged consultants to undertake studies of Large Format Hardware stores. The results of that study for the single large non-metropolitan site (9,948m<sup>2</sup>) are as follows:

### TRAFFIC GENERATION PER 100 M<sup>2</sup> GFA

AM Peak Hour	1.09 vtph
PM Peak Hour	1.99 vtph
Weekend Peak Hour	4.28 vtph

A very pertinent traffic generation characteristic of “large format Hardware” uses is that the larger the floor area the lower the traffic generation rate per 100 m<sup>2</sup> GFA and of the 17,290m<sup>2</sup> proposed some 5,000m<sup>2</sup> will be for building materials storage, landscape yard and trade area.

The assessed potential traffic generation rates for the proposed development having particular regard for the large floor area (ie twice that of the surveyed Bunnings Coffs Harbour and the site surveyed for RMS) are:

WD PM	1.8 vtpm per 100 m <sup>2</sup>
WE MD	3.5 vtpm per 100 m <sup>2</sup>

Application of these rates to the proposed development would indicate the following peak traffic generation:

PM			WEMD		
IN	OUT	TOTAL	IN	OUT	TOTAL
130	180	310	300	300	600

As is normal for significant retail developments there will be a “passing trade” element (ie. vehicles already travelling along the Northern Distributor Road) particularly vehicles travelling to/from the existing Bunnings store to the south. The passing trade will not be as significant as that for a Metropolitan area site (ie. 20 to 30%), however, it is assessed that some 15% will be passing trade on weekdays and some 10% on weekends.

Passing Trade	PMWD	44 vtpm
	WEMD	60 vtpm

The projected directional distribution will be:

East	20%
West	40%
South	40%

In order to take account of future traffic growth (10 years) 10% will be added to the existing major vehicle movements. The projected future peak vehicle movements with the completed development are as follows:

		<b>WDPM</b>	<b>WEMD</b>
Northern Distributor Road	EB	209	141
	RT	153	150
	LT	13	3
	WB	288	105
	RT	40	77
	LT	57	11
Leeds Parade	NB	82	141
	RT	30	16
	LT	295	189
	SB	87	142
	RT	84	134
	LT	39	78

The operational performance of the intersection with these projected future volumes has been modelled using SIDRA and the results indicating a satisfactory performance are provided in Appendix C and summarised in the following:

	<b>WDPM</b>	<b>WEMD</b>
Level of Service	A	A
Av. Vehicle Delay	9.1	9.5

## 6. PARKING

---

Orange City Council's Parking Code specifies a parking provision in relation to the proposed development of 1 space per 50m<sup>2</sup> GFA.

Survey and research of 6 existing Bunnings warehouse stores provides a comprehensive indication of the intrinsic parking demands for Bunnings outlets. The peak demands occur on weekends and it is apparent (as with traffic generation) that the parking demand per 100m<sup>2</sup> generally decreases as the floorspace increases. The established peak parking demand characteristics are summarised in the following:

Thomastown	10,625m <sup>2</sup>	1.37 spaces per 100m <sup>2</sup>
Minchinbury	11,932m <sup>2</sup>	2.0 spaces per 100m <sup>2</sup>
Penrith	13,500m <sup>2</sup>	1.17 spaces per 100m <sup>2</sup>
Hoopers Crossing	11,169m <sup>2</sup>	1.74 spaces per 100m <sup>2</sup>
Mornington	10,599m <sup>2</sup>	2.39 spaces per 100m <sup>2</sup>
Box Hill	13,762m <sup>2</sup>	1.41 spaces per 100m <sup>2</sup>

The recent RMS sponsored study of Non Metropolitan Area Large Format Hardware outlets reveals a maximum parking demand of 1.53 spaces per 100m<sup>2</sup> (ie 1 space per 65m<sup>2</sup>)

The proposed new Orange store of some 17,290m<sup>2</sup> will have 330 parking spaces which equates to 1 space per 52.4m<sup>2</sup>. It is apparent that this provision will be quite adequate, particularly given the significant areas used for storage, and is generally compliant with Council's code. The proposed provision includes 10 suitable and designated spaces for disabled drivers while cars with trailers will be able to park in tandem spaces.

## 7. INTERNAL CIRCULATION AND SERVICING

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### Internal Circulation

The circulation and carpark areas are designed to accord to the design requirements of AS 2890.1 and AS 2890.6 with generous parking bay and aisle provisions. A flexible circulation arrangement will be available throughout the carpark with ability for vehicles to travel to/from either of the external access points.

### Servicing

The Bunnings delivery and service vehicles will ingress on Leeds Parade and travel along the northern side of the building to turn and egress. The Bunnings deliveries will involve semi-trailer and potentially B Double vehicles with some vans and MRV's and the very extensive queuing length available will ensure that there is no possibility on waiting trucks affecting the movement of cars to/from the carpark. Similarly, the provisions in the goods pick-up area will be more than adequate to accommodate demands even at peak seasonal times. Details of the turning path implications of the movements of the largest articulated vehicles are provided in Appendix D indicating a satisfactory provision for access and circulation.

The frequency of delivery vehicle movements for Bunnings will vary from day to day with the projected maximum daily visitation of 1-2 articulated trucks and 2-4 smaller vehicles (HRV and MRV).



## **8. CONCLUSION**

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The proposed Bunnings warehouse development at Orange (replacing the existing store) will utilise the relatively large site which has convenient access to the arterial road system. This assessment has concluded that the design of the development in terms of vehicle access, circulation, parking and servicing is appropriate and that there will not be any adverse traffic impacts on the road system serving the site.

## **Appendix A**

### **AUTOMATIC TRAFFIC SURVEYS**

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## MetroCount Traffic Executive

### Vehicle Counts

#### VehicleCount-385 -- English (ENA)

##### Datasets:

**Site:** [35/08] Leeds Parade north of Honeyman Drive  
**Direction:** 7 - North bound A>B, South bound B>A. Lane: 0  
**Survey Duration:** 8:32 Tuesday, 9 September 2008 => 11:09 Wednesday, 8 October 2008  
**Zone:** .  
**File:** 0808Oct2008.EC0 (Plus)  
**Identifier:** M333VRCW MC56-6 [MC55] (c)Microcom 02/03/01  
**Algorithm:** Factory default (v3.21 - 15275)  
**Data type:** Axle sensors - Paired (Class/Speed/Count)

##### Profile:

**Filter time:** 8:33 Tuesday, 9 September 2008 => 11:09 Wednesday, 8 October 2008  
**Included classes:** 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12  
**Speed range:** 10 - 160 km/h.  
**Direction:** North, East, South, West (bound)  
**Separation:** All - (Headway)  
**Name:** Default Profile  
**Scheme:** Vehicle classification (ARX)  
**Units:** Metric (meter, kilometer, m/s, km/h, kg, tonne)  
**In profile:** Vehicles = 91165 / 91207 (99.95%)



## \* Tuesday, 9 September 2008 - Total=1760 (Incomplete) , 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
-	-	-	-	-	-	-	-	-	0	0	0	0	44	251	340	347	359	160	99	63	44	41	12
-	-	-	-	-	-	-	-	-	0	0	0	0	0	58	75	88	102	52	28	28	8	15	3
-	-	-	-	-	-	-	-	-	0	0	0	0	0	58	97	90	87	41	27	11	13	8	5
-	-	-	-	-	-	-	-	0	0	0	0	0	0	53	86	87	88	36	30	11	11	11	2
-	-	-	-	-	-	-	-	0	0	0	0	0	0	44	82	82	82	31	14	13	12	7	2

PM Peak 1615 - 1715 (361), PM PHF=0.88

## \* Wednesday, 10 September 2008 - Total=3790, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
15	5	8	12	10	38	120	197	373	254	212	243	250	260	295	353	334	348	185	126	57	48	31	16
7	3	1	1	3	1	19	41	73	66	59	50	50	75	63	84	83	95	59	25	20	15	12	5
2	0	1	4	3	4	25	40	98	80	46	63	61	59	95	86	79	87	62	33	10	12	6	4
4	2	2	3	1	12	25	54	97	50	58	68	66	72	65	102	88	102	39	27	15	12	6	3
2	0	4	4	3	21	51	62	105	58	49	62	73	54	72	81	84	64	25	41	12	9	7	4

AM Peak 0800 - 0900 (373), AM PHF=0.89 PM Peak 1645 - 1745 (368), PM PHF=0.90

## \* Thursday, 11 September 2008 - Total=3970, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
19	20	8	10	12	44	150	203	388	303	232	229	234	253	269	352	382	354	205	113	93	50	31	15
1	9	1	0	1	5	25	44	73	85	59	57	63	69	72	82	119	96	51	43	37	17	13	6
6	4	1	3	5	6	33	43	93	83	46	62	54	65	67	93	82	92	56	27	17	16	7	3
6	3	0	5	3	13	25	48	101	64	66	48	50	59	68	89	102	95	48	17	17	11	6	4
6	4	6	2	3	20	67	68	121	71	62	62	67	60	62	88	79	71	50	26	22	6	5	2

AM Peak 0815 - 0915 (400), AM PHF=0.83 PM Peak 1545 - 1645 (391), PM PHF=0.82

## \* Friday, 12 September 2008 - Total=4004, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
11	13	7	8	7	32	122	217	350	313	247	235	277	280	310	353	326	330	225	112	103	64	36	26
1	3	1	4	0	4	16	46	82	98	73	65	72	62	68	78	87	91	77	42	22	17	10	9
3	5	0	1	2	6	22	45	79	77	50	52	76	76	73	89	73	86	60	26	36	20	12	6
4	3	3	2	3	8	24	59	90	57	61	58	62	73	89	81	87	90	48	20	25	14	9	7
3	2	3	1	2	14	60	67	99	81	63	60	67	69	80	105	79	63	40	24	20	13	5	4

AM Peak 0815 - 0915 (366), AM PHF=0.92 PM Peak 1515 - 1615 (362), PM PHF=0.86

## \* Saturday, 13 September 2008 - Total=2724, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
15	12	9	4	8	16	68	92	201	252	284	230	228	189	204	208	172	149	127	81	59	41	36	39
5	2	5	1	1	3	15	14	34	64	67	66	61	49	60	52	42	35	39	28	12	10	11	8
3	3	0	0	1	3	9	23	42	53	61	55	53	46	50	45	44	41	33	18	17	9	8	15
4	4	2	2	4	4	16	25	47	66	73	60	62	37	39	58	39	41	34	17	16	14	11	9
3	3	2	1	2	6	28	30	78	69	83	49	52	57	55	53	47	32	21	18	14	8	6	7

AM Peak 1000 - 1100 (284), AM PHF=0.86 PM Peak 1200 - 1300 (228), PM PHF=0.92

## \* Sunday, 14 September 2008 - Total=1897, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
6	7	4	4	7	11	14	37	58	126	182	179	187	144	196	197	162	138	95	60	43	23	10	7
2	2	0	1	2	2	3	9	9	23	42	38	70	42	53	48	43	34	36	25	12	8	2	1
2	4	1	2	1	2	3	7	14	33	47	34	35	29	48	50	44	40	24	10	10	3	3	2
1	0	2	0	3	5	5	6	16	36	43	40	53	39	52	61	41	31	24	13	14	5	3	3
1	1	1	1	1	2	3	15	19	34	50	67	29	34	43	38	34	33	11	12	7	7	2	1

AM Peak 1145 - 1245 (225), AM PHF=0.80 PM Peak 1445 - 1545 (202), PM PHF=0.83

## \* Monday, 15 September 2008 - Total=3251, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
4	1	4	6	8	48	123	213	348	242	191	175	228	213	208	279	302	295	159	81	56	43	16	8
2	0	1	0	2	2	21	33	67	92	40	46	53	52	50	47	67	81	51	19	24	14	3	3
1	0	1	1	3	10	24	55	87	50	46	34	54	50	39	71	76	81	46	23	9	16	5	2
0	1	0	2	2	13	22	52	93	44	50	37	58	58	49	82	81	69	24	20	14	8	6	1
1	0	2	3	1	23	56	73	101	56	55	58	63	53	70	79	78	64	38	19	9	5	2	2

AM Peak 0815 - 0915 (373), AM PHF=0.92 PM Peak 1630 - 1730 (321), PM PHF=0.99

## \* Tuesday, 16 September 2008 - Total=3478, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
6	2	4	10	10	47	131	209	349	267	228	198	209	229	202	290	324	329	186	89	85	33	28	13
3	0	0	1	3	3	19	46	86	86	48	62	47	61	45	54	85	91	49	24	33	3	11	3
1	0	1	3	3	9	24	43	88	70	56	47	56	68	45	72	74	94	63	17	13	15	3	5
1	1	1	2	2	11	30	55	89	55	58	47	56	51	48	83	79	76	45	27	25	7	7	4
1	1	2	4	2	24	58	65	86	56	66	42	50	49	64	81	86	68	29	21	14	8	7	1

AM Peak 0800 - 0900 (349), AM PHF=0.98 PM Peak 1630 - 1730 (350), PM PHF=0.93

## \* Wednesday, 17 September 2008 - Total=3534, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
10	6	5	10	3	46	132	210	354	270	244	175	235	245	189	286	353	336	180	113	56	46	24	6
0	4	1	2	1	1	22	34	97	85	59	36	41	69	45	70	85	110	55	29	18	13	7	3
8	2	0	2	0	9	17	54	72	73	62	37	47	57	46	71	88	91	49	26	16	15	4	2
1	0	1	2	1	13	28	44	116	60	61	40	72	53	47	71	88	68	45	34	11	4	6	0
1	0	3	4	1	23	65	78	69	52	62	62	75	66	51	74	92	67	31	24	11	14	7	1

AM Peak 0745 - 0845 (363), AM PHF=0.78 PM Peak 1630 - 1730 (381), PM PHF=0.87

## \* Thursday, 18 September 2008 - Total=3667, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
8	5	4	7	9	41	118	204	355	270	245	210	225	241	285	309	355	327	190	112	49	54	27	17
4	2	3	0	0	3	20	52	78	88	47	60	56	74	76	68	96	84	62	34	15	19	8	7
1	0	0	3	1	9	21	51	87	57	70	51	62	53	73	81	99	84	39	23	12	16	8	3
1	2	0	1	4	10	23	37	90	69	66	42	54	58	67	76	81	84	46	28	11	9	7	3
2	1	1	3	4	19	54	64	100	56	62	57	53	56	69	84	79	75	43	27	11	10	4	4

AM Peak 0815 - 0915 (365), AM PHF=0.91 PM Peak 1545 - 1645 (360), PM PHF=0.91



**\* Friday, 19 September 2008 - Total=3712, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
9	6	7	4	11	39	127	225	360	250	226	254	232	274	265	334	353	280	191	87	60	58	45	15
3	0	1	0	3	4	21	36	71	65	51	68	41	63	56	78	92	71	65	29	15	11	15	6
2	1	2	0	1	8	25	55	95	48	59	64	58	77	80	74	81	78	44	21	12	14	12	3
3	3	3	3	3	7	25	71	93	66	66	58	52	64	58	95	94	73	47	18	22	21	9	3
1	2	1	1	4	20	56	63	101	71	50	64	81	70	71	87	86	58	35	19	11	12	9	3

AM Peak 0800 - 0900 (360), AM PHF=0.89 PM Peak 1530 - 1630 (355), PM PHF=0.93

**\* Saturday, 20 September 2008 - Total=2556, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
21	10	6	6	5	21	48	102	165	214	251	234	215	204	203	179	153	162	101	91	55	47	48	15
8	4	2	0	1	2	6	16	32	50	58	48	49	54	46	44	39	37	37	25	13	12	11	6
7	4	1	2	2	6	10	23	39	51	64	65	53	46	53	53	39	52	22	24	18	10	10	3
2	2	1	2	2	5	12	29	37	47	57	63	58	50	51	42	35	32	26	18	11	12	12	4
4	0	2	2	0	8	20	34	57	66	72	58	55	54	53	40	40	41	16	24	13	13	15	2

AM Peak 1000 - 1100 (251), AM PHF=0.87 PM Peak 1215 - 1315 (220), PM PHF=0.95

**\* Sunday, 21 September 2008 - Total=2497, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
14	8	8	5	1	5	25	47	102	159	203	207	260	235	222	239	238	187	130	74	64	31	22	11
3	6	4	2	0	2	6	10	24	38	41	58	61	47	59	68	69	55	43	20	13	10	11	2
4	0	1	2	0	2	6	8	27	32	42	44	64	54	52	60	61	41	36	30	18	5	4	3
3	2	1	1	0	0	4	12	27	42	53	48	67	58	49	59	57	52	33	16	18	11	3	3
4	0	2	0	1	1	9	17	24	47	67	57	68	76	62	52	51	39	18	8	15	5	4	3

AM Peak 1145 - 1245 (249), AM PHF=0.93 PM Peak 1200 - 1300 (260), PM PHF=0.96

**\* Monday, 22 September 2008 - Total=3292, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
3	3	6	3	5	39	133	220	364	277	197	237	231	193	209	294	309	274	133	65	32	30	24	11
1	1	4	0	2	5	24	33	64	96	45	55	48	49	44	53	85	85	50	25	13	8	6	4
1	0	1	1	1	8	23	43	97	66	44	57	63	42	38	70	82	57	29	18	10	5	8	3
0	1	0	0	2	10	28	61	95	63	50	65	54	53	59	86	77	73	29	10	4	7	4	1
1	1	1	2	0	16	58	83	108	52	58	60	66	49	68	85	65	59	25	12	5	10	6	3

AM Peak 0815 - 0915 (396), AM PHF=0.92 PM Peak 1530 - 1630 (338), PM PHF=0.98

**\* Tuesday, 23 September 2008 - Total=3364, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
5	5	4	10	5	49	132	175	350	253	222	214	214	207	227	314	329	287	149	87	49	42	23	12
2	3	0	0	1	7	21	38	68	79	52	52	42	51	51	84	92	84	45	32	16	12	9	6
2	0	3	6	0	13	20	33	82	68	54	42	62	43	43	72	80	82	54	22	13	8	4	2
0	1	0	2	3	13	37	40	96	52	49	62	54	56	65	93	89	61	29	18	14	12	4	3
1	1	1	2	1	16	54	64	104	54	67	58	56	57	68	65	68	60	21	15	6	10	6	1

AM Peak 0815 - 0915 (361), AM PHF=0.87 PM Peak 1530 - 1630 (330), PM PHF=0.89

**\* Wednesday, 24 September 2008 - Total=3614, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
7	5	6	7	8	54	120	236	315	249	204	228	238	240	250	315	352	327	170	106	79	52	35	11
2	2	0	0	2	5	17	48	63	61	50	45	64	79	64	82	94	81	59	36	25	13	10	5
1	0	3	1	0	13	17	53	78	61	38	56	69	53	63	70	80	93	53	28	18	13	12	4
3	2	1	3	3	11	26	62	85	59	58	56	47	52	63	93	90	69	38	29	22	11	6	1
1	1	2	3	3	25	60	73	89	68	58	71	58	56	60	70	88	84	20	13	14	15	7	1

AM Peak 0800 - 0900 (315), AM PHF=0.88 PM Peak 1600 - 1700 (352), PM PHF=0.94

**\* Thursday, 25 September 2008 - Total=3506, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
6	3	5	5	7	49	122	193	379	254	203	195	204	225	230	334	342	302	182	117	79	29	29	12
2	1	2	2	2	6	15	44	69	85	48	48	44	60	57	70	93	101	68	37	26	8	9	5
1	0	0	1	0	9	22	52	98	60	53	58	67	54	59	88	81	68	39	33	18	8	8	5
0	1	3	1	2	10	20	40	100	53	45	51	48	66	51	98	86	66	42	30	18	9	5	1
3	1	0	1	3	24	65	57	112	56	57	38	45	45	63	78	82	67	33	17	17	4	7	1

AM Peak 0815 - 0915 (395), AM PHF=0.88 PM Peak 1515 - 1615 (357), PM PHF=0.91

**\* Friday, 26 September 2008 - Total=3815, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
9	8	3	5	6	47	138	237	352	288	264	249	227	275	265	340	316	275	186	115	97	42	46	25
4	2	1	3	0	3	25	46	69	72	59	60	55	76	56	90	101	79	63	28	27	8	10	6
5	4	0	0	2	12	22	51	91	78	64	62	50	50	70	87	82	67	56	28	17	17	6	5
0	1	0	1	1	8	30	59	98	72	60	59	62	67	73	85	68	62	35	32	31	10	18	4
0	1	2	1	3	24	61	81	94	66	81	68	60	82	66	78	65	67	32	27	22	7	12	10

AM Peak 0815 - 0915 (355), AM PHF=0.91 PM Peak 1515 - 1615 (351), PM PHF=0.87

**\* Saturday, 27 September 2008 - Total=2572, 15 minute drops**

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300
18	12	8	10	5	12	52	99	153	186	265	252	239	184	188	175	187	158	130	74	42	50	43	30
7	2	0	4	0	2	7	23	30	44	72	55	65	54	38	45	57	36	37	29	12	8	13	7
5	5	1	3	2	4	4	20	29	50	60	71	56	40	48	43	42	44	31	15	8	17	12	9
5	3	2	2	2	14	30	54	46	65	55	63	44	44	52	41	44	39	30	15	11	14	11	8
1	2	5	1	1	4	27	26	40	46	68	71	55	46	50	46	44	39	32	15	11	11	7	6

AM Peak 1000 - 1100 (265), AM PHF=0.92 PM Peak 1200 - 1300 (239), PM PHF=0.92

**\* Sunday, 28 September 2008 - Total=2193, 15 minute drops**

Sunday, 28 September 2008																								Total: 2150, 10 minutes drop	
0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300		
14	11	1	6	7	8	25	54	94	137	164	227	199	194	209	199	189	159	119	77	31	35	18	16		
7	5	0	3	3	1	6	16	35	34	64	55	39	39	54	56	54	37	44	24	9	11	6	6	1	
2	3	1	1	3	1	6	5	22	24	47	59	53	49	38	56	49	45	35	17	6	9	8	5	1	
5	2	0	0	1	3	8	21	32	31	37	51	49	55	58	39	48	34	22	20	14	7	2	3	1	
0	1	0	2	0	3	10	22	24	47	46	53	42	51	59	48	38	43	18	16	2	8	2	2	0	







## **Appendix B**

### **INTERSECTION SURVEYS**

---



# R.O.A.R. DATA

Reliable, Original & Authentic Results

Ph.88196847, Fax 88196849, Mob.0418-239019

Client

: T.T.P.A.

Job No/Name

: 4578 ORANGE Leeds Pde

Day/Date

: Friday 5th April 2013

	NORTH				NORTHEAST				EAST				SOUTH				WEST				
	Leeds Pde				Bus Depot				Northern Distributor				Leeds Pde				Northern Distributor				
	R	I	L	HL	HR	R	I	L	HR	R	I	L	HR	R	I	L	HR	R	I	L	HL
1500 - 1515	2	7	4	0	0	1	1	0	0	1	39	5	14	5	6	25	23	32	0	0	165
1515 - 1530	5	9	2	1	0	1	0	1	0	1	48	15	7	0	8	47	35	45	0	1	226
1530 - 1545	5	7	4	0	0	0	1	0	1	0	55	10	9	0	5	60	50	38	1	0	246
1545 - 1600	5	9	7	0	0	1	1	0	0	1	23	8	12	3	6	77	32	41	1	7	234
1600 - 1615	9	8	2	0	0	1	0	0	0	1	50	9	8	3	3	78	50	55	5	5	287
1615 - 1630	7	10	2	0	0	2	1	1	0	2	50	10	8	0	3	55	45	32	5	3	236
1630 - 1645	5	7	2	1	0	2	1	1	0	0	50	10	11	0	2	57	32	57	2	3	243
1645 - 1700	5	6	0	0	0	2	3	1	0	0	73	9	9	5	7	77	36	61	2	5	301
1700 - 1715	14	17	7	0	0	5	2	2	0	0	94	23	2	3	1	82	30	45	2	2	331
1715 - 1730	5	10	3	0	0	3	2	1	0	0	43	7	1	0	0	65	41	39	2	0	222
1730 - 1745	7	2	1	0	0	3	5	0	0	0	53	5	3	1	5	64	25	27	2	0	203
1745 - 1800	5	7	1	0	0	2	0	0	1	0	47	3	6	5	3	41	28	47	0	1	197
Period End	74	99	35	2	0	23	17	7	2	6	625	114	90	25	49	728	427	519	22	27	2891

	NORTH				NORTHEAST				EAST				SOUTH				WEST				
	Leeds Pde				Bus Depot				Northern Distributor				Leeds Pde				Northern Distributor				
	R	I	L	HL	HR	R	I	L	HR	R	I	L	HR	R	I	L	HR	R	I	L	HL
1500 - 1600	17	32	17	1	0	3	3	1	1	3	165	38	42	8	25	209	140	156	2	8	871
1515 - 1615	24	33	15	1	0	3	2	1	1	3	176	42	36	6	22	262	167	179	7	13	993
1530 - 1630	26	34	15	0	0	4	3	1	1	4	178	37	37	6	17	270	177	166	12	15	1003
1545 - 1645	26	34	13	1	0	6	3	2	0	4	173	37	39	6	14	267	159	185	13	18	1000
1600 - 1700	26	31	6	1	0	7	5	3	0	3	223	38	36	8	15	267	163	205	14	16	1067
1615 - 1715	31	40	11	1	0	11	7	5	0	2	267	52	30	8	13	271	143	195	11	13	1111
1630 - 1730	29	40	12	1	0	12	8	5	0	0	260	49	23	8	10	281	139	202	8	10	1097
1645 - 1745	31	35	11	0	0	13	12	4	0	0	263	44	15	9	13	288	132	172	8	7	1057
1700 - 1800	31	36	12	0	0	13	9	3	1	0	237	38	12	9	9	252	124	158	6	3	953

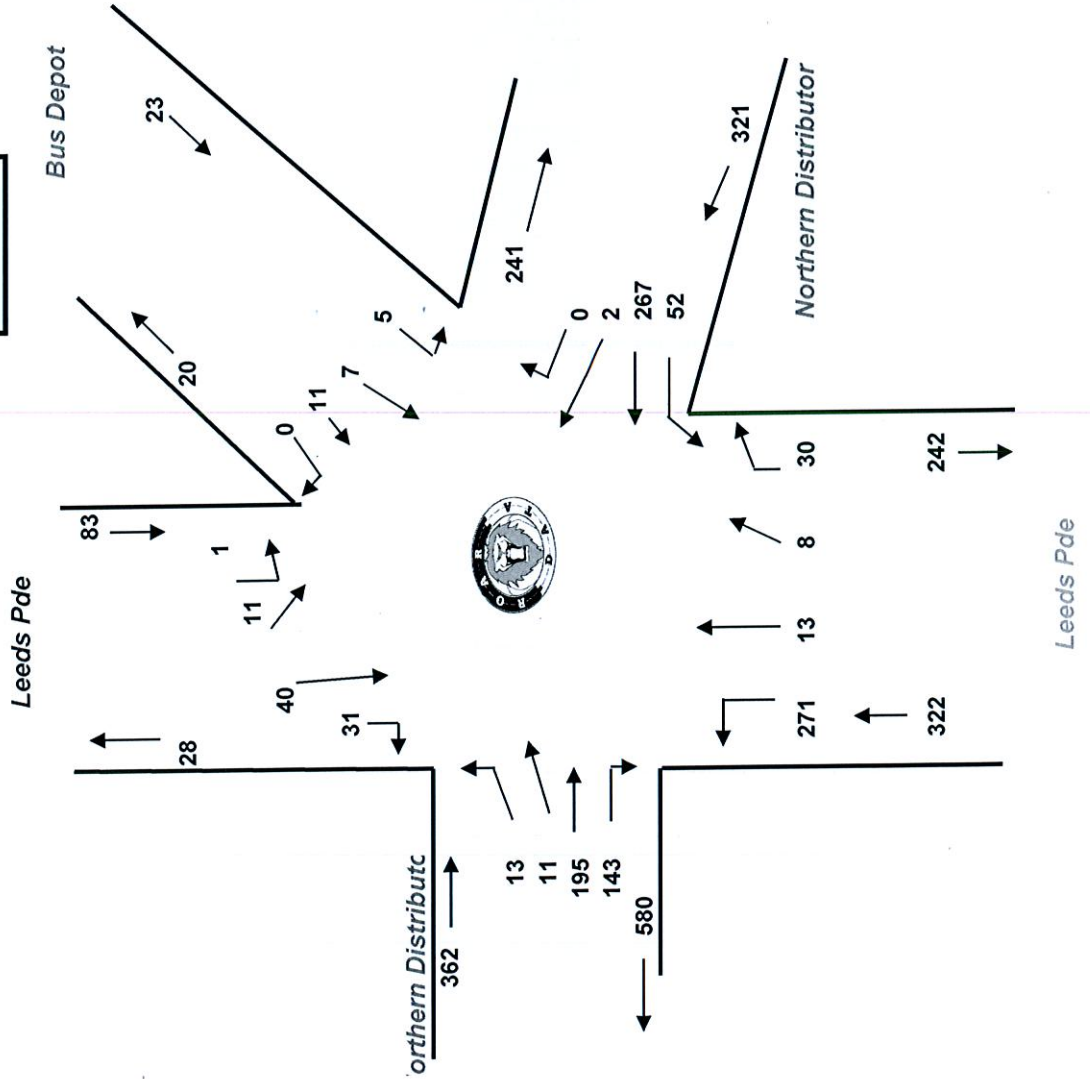
PEAK HOUR	31	40	11	1	0	11	7	5	0	0	2	267	52	30	8	13	271	143	195	11	13	1111
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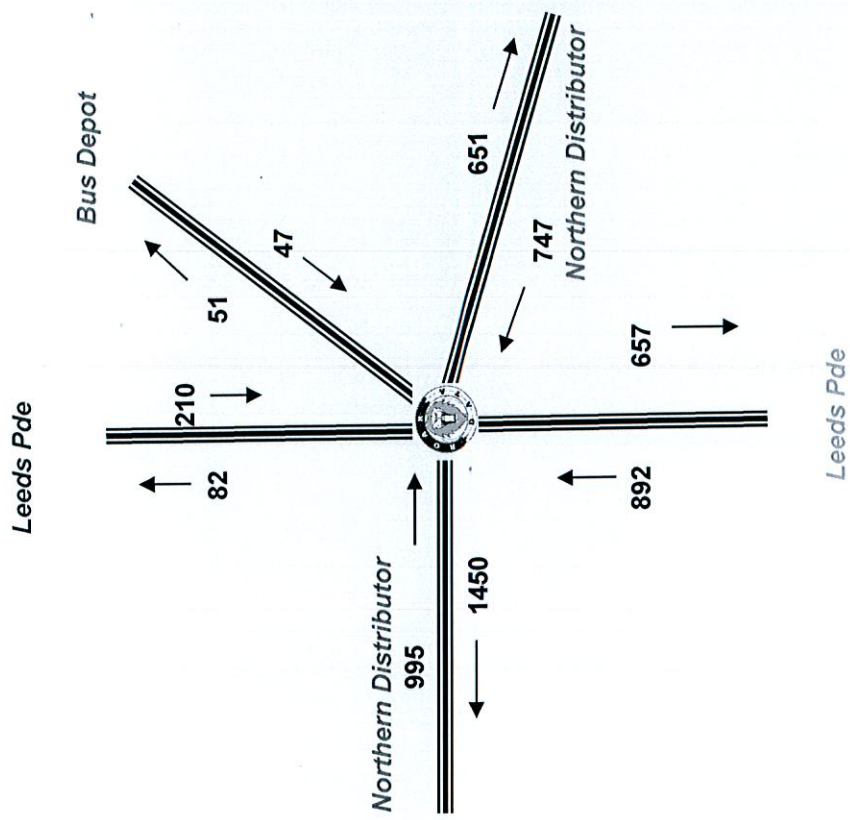
**R.O.A.R. DATA**  
Reliable, Original & Authentic Results  
Ph. 88196847, Fax 88196849, Mob. 0418-239019

Client : T.T.P.A.  
Job No/Name : 4578 ORANGE Leeds Pde  
Day/Date : Friday 5th April 2013

**PEAK HOUR**  
1615 - 1715



**TOTAL VOLUMES  
FOR PERIODS  
COUNTED**







# R.O.A.R. DATA

Reliable, Original & Authentic Results

Ph.88196847, Fax 88196849, Mob.0418-239019

Client

: T.T.P.A.

Job No/Name

: 4578 ORANGE Leeds Pde

Day/Date

: Saturday 6th April 2013

Time Per	NORTH					NORTHEAST					EAST					SOUTH					WEST					
	Leeds Pde					Bus Depot					Northern Distributor					Leeds Pde					Northern Distributor					
	R	I	L	HL		HR	R	I	L		HR	R	I	L		HR	R	I	L		R	I	L	HL	TOT	
1100 - 1115	3	5	0	0		0	1	0	1		0	0	0	28	4	3	0	2	33		33	31	1	3		148
1115 - 1130	2	2	0	0		0	1	0	2		0	1	26	3		1	2	2	36		33	34	0	1		146
1130 - 1145	2	4	0	0		0	1	0	2		1	0	20	0		4	0	1	41		25	29	2	0		132
1145 - 1200	1	2	0	0		0	2	0	0		0	0	32	3		5	0	0	46		18	24	0	0		133
1200 - 1215	0	1	1	0		0	0	0	0		0	0	30	3		8	1	3	36		45	42	0	2		172
1215 - 1230	0	2	0	0		0	0	0	0		0	0	32	3		5	0	4	55		30	41	2	1		175
1230 - 1245	1	4	1	0		0	0	2	0		0	0	33	3		1	0	0	53		32	34	0	0		164
1245 - 1300	1	3	0	0		0	1	0	0		0	1	18	2		2	0	2	46		38	20	0	0		134
1300 - 1315	1	1	1	0		0	1	3	0		0	0	30	3		2	2	2	51		29	28	2	2		158
1315 - 1330	3	3	0	0		0	2	1	1		0	0	35	3		5	1	3	36		27	29	0	0		149
1330 - 1345	0	3	0	0		0	1	0	1		0	0	30	3		0	0	2	55		32	37	1	3		168
1345 - 1400	0	3	0	0		0	0	0	0		0	0	48	4		4	3	1	34		30	30	1	2		160
Period End	14	33	3	0		0	10	6	7		1	2	362	34		40	9	22	522		372	379	9	14		1839

	NORTH				NORTHEAST					EAST					SOUTH					WEST				
	Leeds Pde				Bus Depot					Northern Distributor					Leeds Pde					Northern Distributor				
	<u>R</u>	<u>I</u>	<u>L</u>	<u>HL</u>	<u>HR</u>	<u>R</u>	<u>I</u>	<u>L</u>	<u>HR</u>	<u>R</u>	<u>I</u>	<u>L</u>	<u>HR</u>	<u>R</u>	<u>I</u>	<u>L</u>	<u>HR</u>	<u>R</u>	<u>I</u>	<u>L</u>	<u>HL</u>	TOT		
1100 - 1200	8	13	0	0	0	5	0	5	1	1	106	10	13	2	5	156	109	118	3	4	559			
1115 - 1215	5	9	1	0	0	4	0	4	1	1	108	9	18	3	6	159	121	129	2	3	583			
1130 - 1230	3	9	1	0	0	3	0	2	1	0	114	9	22	1	8	178	118	136	4	3	612			
1145 - 1245	2	9	2	0	0	2	2	0	0	0	127	12	19	1	7	190	125	141	2	3	644			
1200 - 1300	2	10	2	0	0	1	2	0	0	1	113	11	16	1	9	190	145	137	2	3	645			
1215 - 1315	3	10	2	0	0	2	5	0	0	1	113	11	10	2	8	205	129	123	4	3	631			
1230 - 1330	6	11	2	0	0	4	6	1	0	1	116	11	10	3	7	186	126	111	2	2	605			
1245 - 1345	5	10	1	0	0	5	4	2	0	1	113	11	9	3	9	188	126	114	3	5	609			
1300 - 1400	4	10	1	0	0	4	4	2	0	0	143	13	11	6	8	176	118	124	4	7	635			

PEAK HOUR	2	10	2	0		0	1	2	0		0	1	113	11		16	1	9	190		145	137	2	3		645
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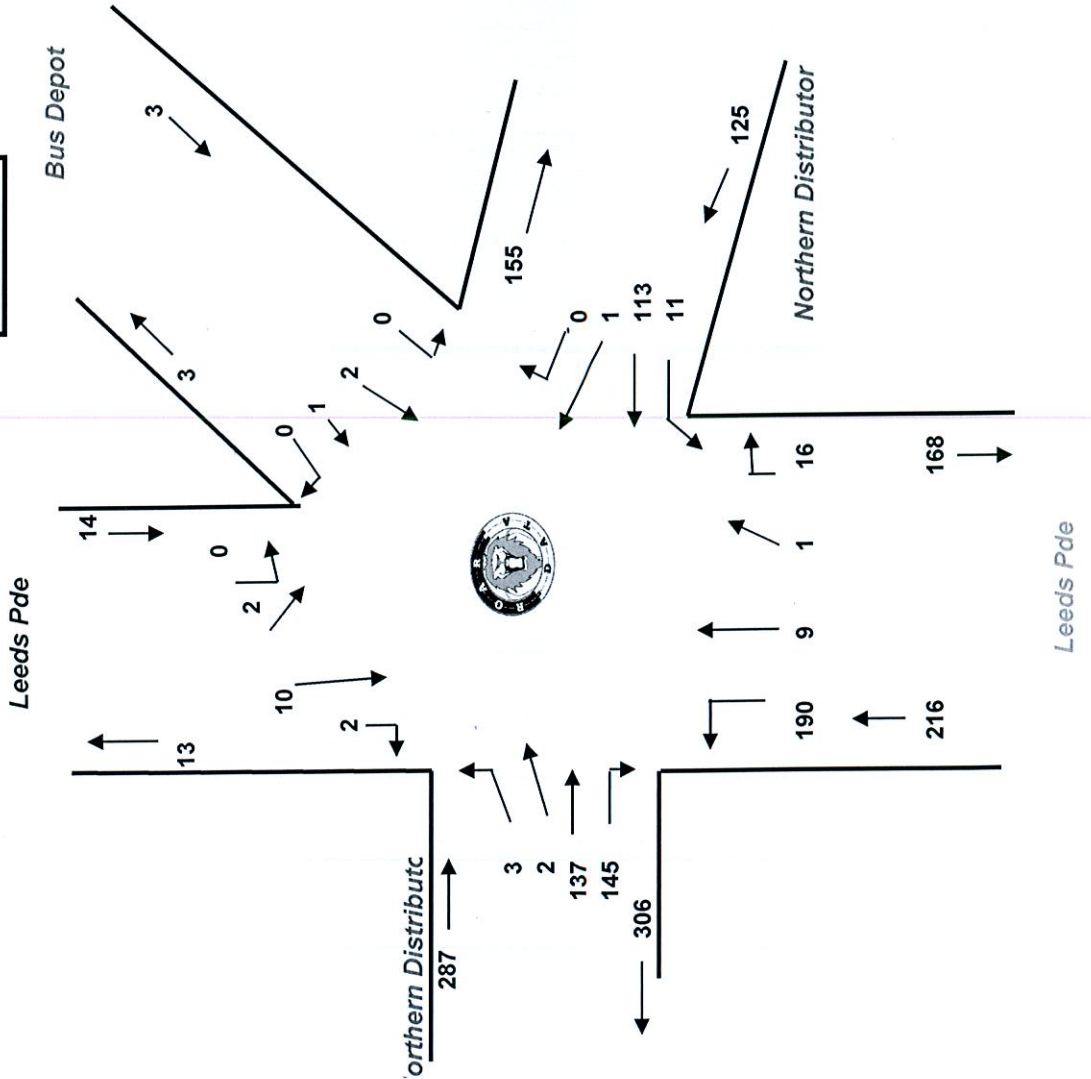
# R.O.A.R. DATA

Reliable, Original & Authentic Results

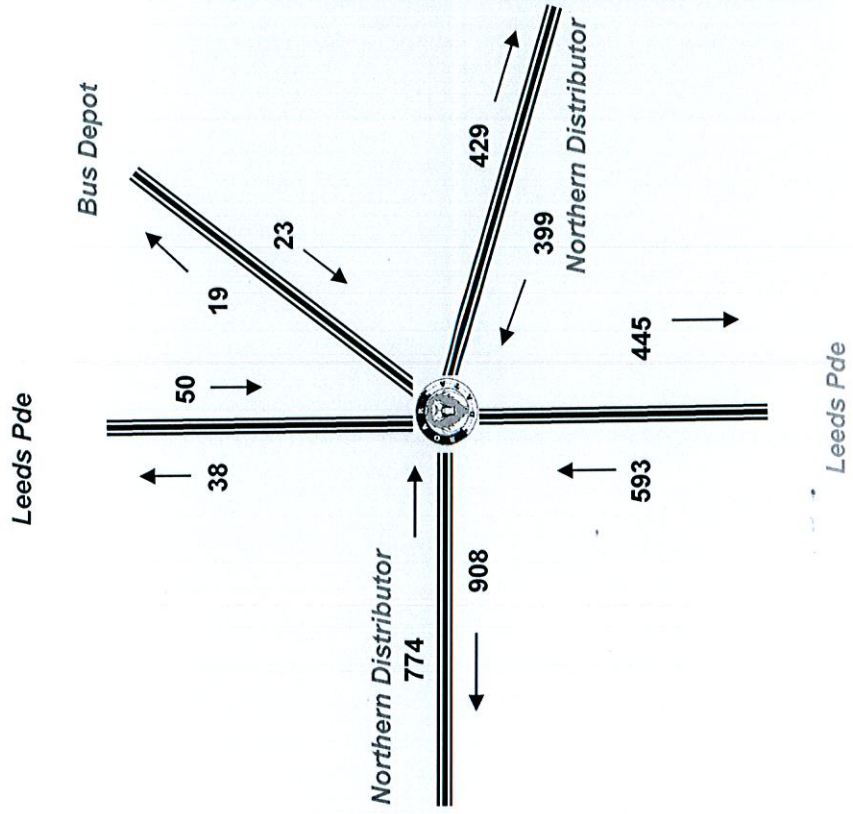
Ph.88196847, Fax 88196849, Mob.0418-239019

Client : T.T.P.A.  
Job No/Name : 4578 ORANGE Leeds Pde  
Day/Date : Saturday 6th April 2013

PEAK HOUR  
1200 - 1300



TOTAL VOLUMES  
FOR PERIODS  
COUNTED



## **Appendix C**

### **SIDRA RESULTS**

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# MOVEMENT SUMMARY

Site: Northern Distributor WD PM

Roundabout

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Leeds Pde											
1	L	311	5.0	0.256	7.3	LOS A	1.3	9.2	0.49	0.56	55.0
2	T	86	5.0	0.152	8.1	LOS A	0.6	4.9	0.49	0.61	54.4
3	R	40	25.0	0.152	17.3	LOS B	0.6	4.9	0.49	0.90	48.7
Approach		437	6.8	0.256	8.3	LOS A	1.3	9.2	0.49	0.60	54.2
South East: Northern Dist											
21	L	60	5.0	0.049	8.0	LOS A	0.2	1.1	0.30	0.54	54.9
22	T	303	5.0	0.157	6.9	LOS A	0.8	5.6	0.40	0.53	55.5
23	R	42	5.0	0.157	13.1	LOS A	0.8	5.6	0.40	0.81	51.5
Approach		405	5.0	0.157	7.7	LOS A	0.8	5.6	0.39	0.56	55.0
North East: Bus Depot											
24	L	13	100.0	0.072	16.8	LOS B	0.2	3.2	0.59	0.77	48.1
26	R	12	100.0	0.072	24.2	LOS B	0.2	3.2	0.59	0.92	45.1
Approach		24	100.0	0.072	20.4	LOS B	0.2	3.2	0.59	0.84	46.6
North: Leeds Pde											
7	L	29	8.4	0.063	7.9	LOS A	0.2	1.8	0.45	0.60	55.3
8	T	81	5.0	0.119	7.3	LOS A	0.5	3.6	0.44	0.55	54.8
9	R	88	5.0	0.119	16.2	LOS B	0.5	3.6	0.43	0.81	48.9
Approach		199	5.5	0.119	11.3	LOS A	0.5	3.6	0.44	0.67	51.9
North West: Northern Dist											
27	L	25	48.5	0.154	10.2	LOS A	0.7	5.2	0.33	0.64	54.9
28	T	233	5.0	0.195	6.7	LOS A	1.0	7.0	0.32	0.48	56.1
29	R	172	5.0	0.195	12.8	LOS A	1.0	7.0	0.31	0.72	51.0
Approach		429	7.6	0.195	9.3	LOS A	1.0	7.0	0.32	0.58	53.8
All Vehicles		1495	7.9	0.256	9.1	LOS A	1.3	9.2	0.41	0.60	53.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

Processed: Monday, 29 April 2013 9:50:06 AM

SIDRA INTERSECTION 5.1.13.2093

Project: F:\WORK\12\12084 - BUNNINGS ORANGE\MODELLING\Bunnings Orange.sip

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**SIDRA**  
**INTERSECTION**



# MOVEMENT SUMMARY

Site: Northern Distributor WE MD

Roundabout

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Seg Sat v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Leeds Pde											
1	L	199	5.0	0.155	6.9	LOS A	0.7	5.1	0.39	0.53	55.8
2	T	148	5.0	0.149	7.1	LOS A	0.7	4.8	0.41	0.55	55.5
3	R	18	10.6	0.149	16.2	LOS B	0.7	4.8	0.41	0.90	49.6
Approach		365	5.3	0.155	7.4	LOS A	0.7	5.1	0.40	0.56	55.3
South East: Northern Dist											
21	L	12	5.0	0.009	8.1	LOS A	0.0	0.2	0.33	0.53	54.7
22	T	111	5.0	0.091	7.2	LOS A	0.4	3.2	0.44	0.54	54.9
23	R	81	5.0	0.091	13.3	LOS A	0.4	3.2	0.44	0.74	50.7
Approach		203	5.0	0.091	9.6	LOS A	0.4	3.2	0.43	0.62	53.1
North East: Bus Depot											
24	L	2	100.0	0.010	16.3	LOS B	0.0	0.4	0.59	0.66	48.7
26	R	1	100.0	0.010	24.4	LOS B	0.0	0.4	0.59	0.85	45.4
Approach		3	100.0	0.010	19.0	LOS B	0.0	0.4	0.59	0.73	47.5
North: Leeds Pde											
7	L	51	5.0	0.101	7.4	LOS A	0.4	2.9	0.43	0.57	55.5
8	T	139	5.0	0.189	7.1	LOS A	0.8	6.2	0.43	0.54	54.9
9	R	141	5.0	0.189	16.1	LOS B	0.8	6.2	0.42	0.81	49.0
Approach		331	5.0	0.189	11.0	LOS A	0.8	6.2	0.43	0.66	52.1
North West: Northern Dist											
27	L	5	43.0	0.134	10.2	LOS A	0.6	4.4	0.37	0.67	54.8
28	T	180	5.0	0.169	6.9	LOS A	0.8	5.9	0.36	0.51	55.8
29	R	189	5.0	0.169	13.1	LOS A	0.8	5.9	0.36	0.70	50.5
Approach		375	5.5	0.169	10.1	LOS A	0.8	5.9	0.36	0.61	52.9
All Vehicles		1277	5.5	0.189	9.5	LOS A	0.8	6.2	0.40	0.61	53.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

Processed: Monday, 29 April 2013 9:55:27 AM

SIDRA INTERSECTION 5.1.13.2093

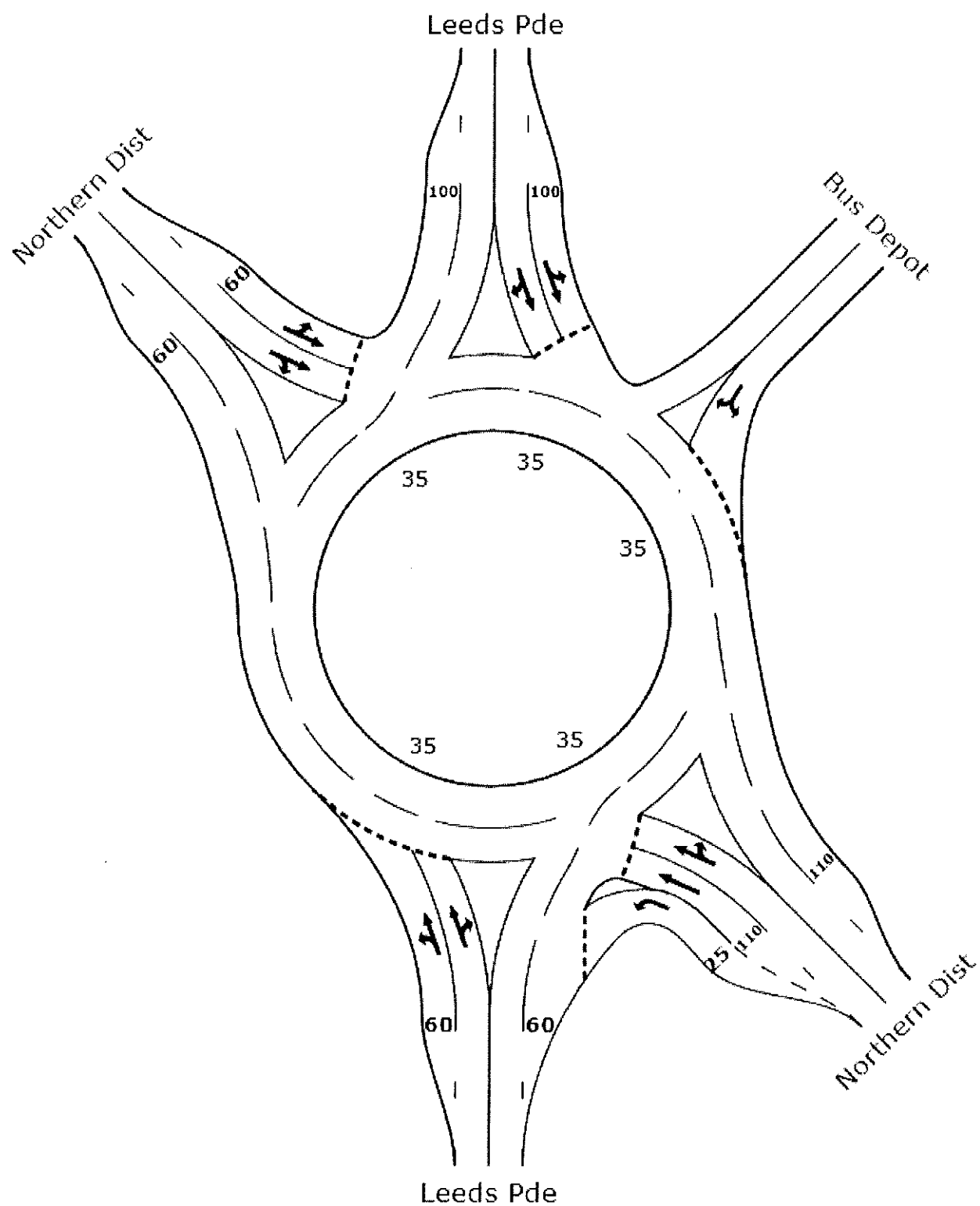
Project: F:\WORK\12\12084 - BUNNINGS ORANGE\MODELLING\Bunnings Orange.sip

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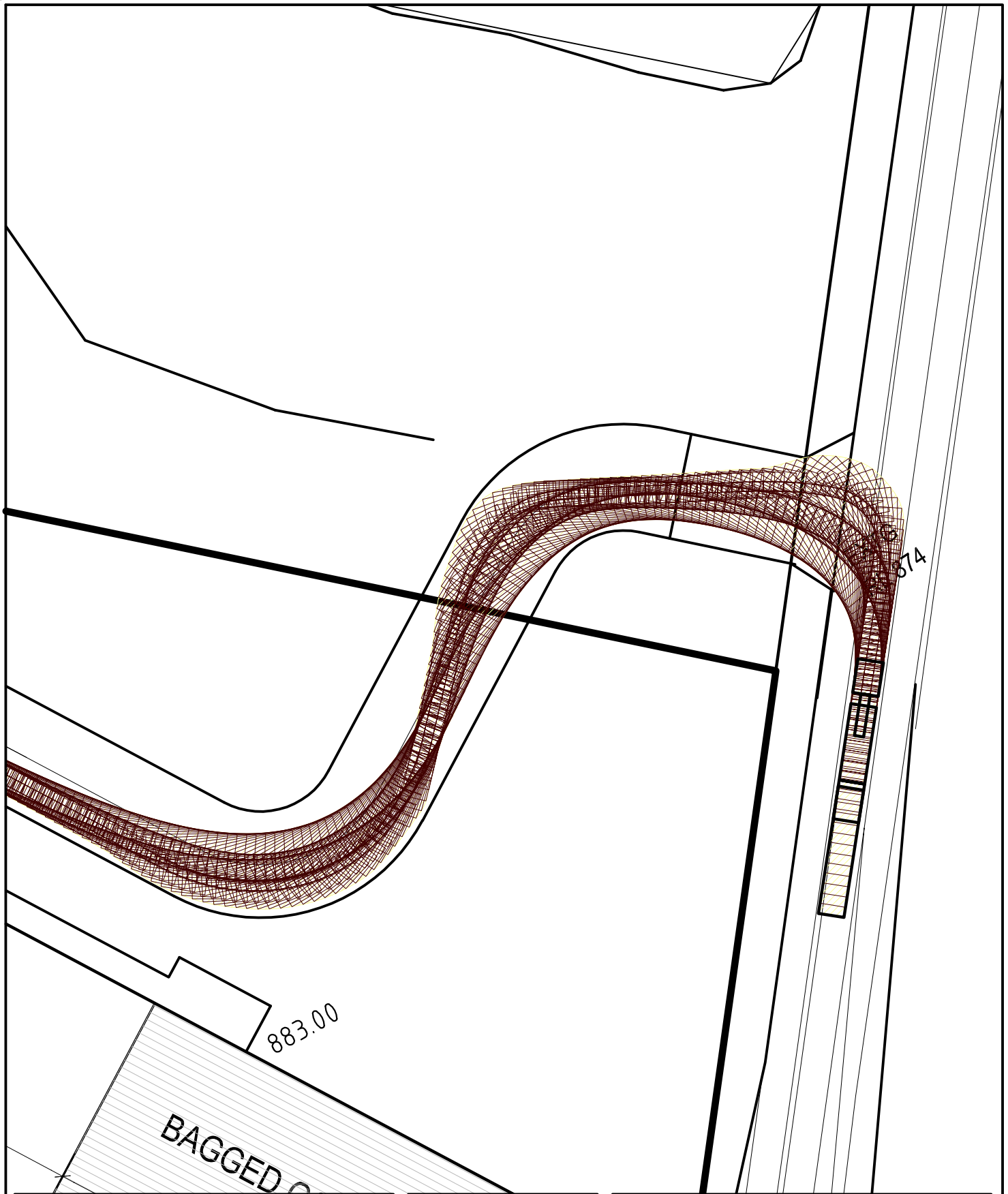
SIDRA  
INTERSECTION



## **Appendix D**

### **TURNING PATH ASSESSMENT**

---



## LEGEND

This drawing has been prepared using vehicle modelling computer software AutoTrack V9.21 in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS  
OF A 25m ARTICULATED  
VEHICLE ENTERING THE SITE**

**SP 1**



## LEGEND

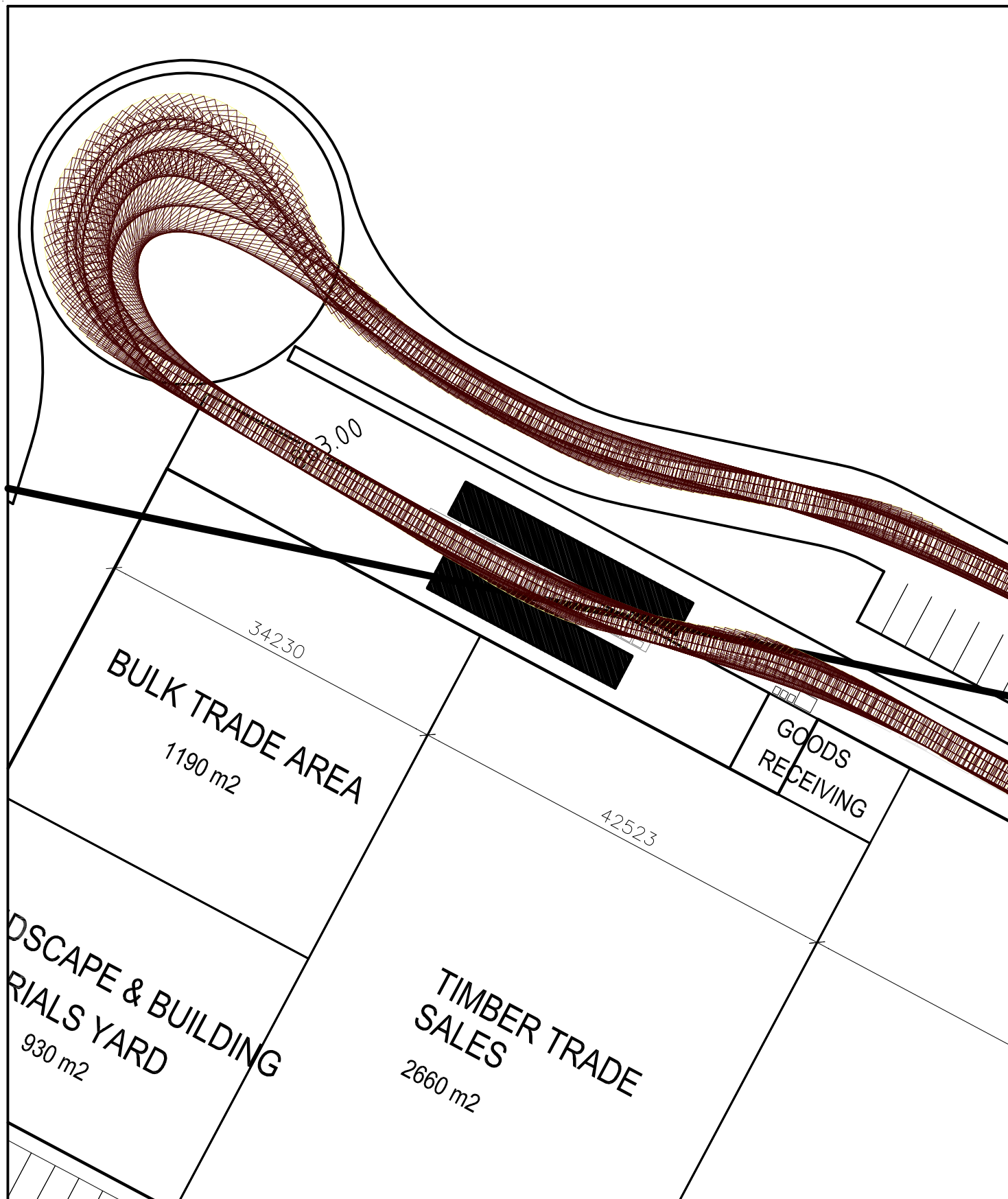
This drawing has been prepared using vehicle modelling computer software AutoTrack V9.21 in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS  
OF A 25m ARTICULATED  
VEHICLE**

**SP 2**





## LEGEND

This drawing has been prepared using vehicle modelling computer software AutoTrack V9.21 in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS  
OF A 25m ARTICULATED  
VEHICLE**

**SP 3**



## LEGEND

This drawing has been prepared using vehicle modelling computer software AutoTrack V9.21 in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS  
OF A 25m ARTICULATED  
VEHICLE EXITING THE SITE**

**SP 4**

# **Appendix B**

---

## **SIDRA ANALYSIS DATA**

## DETAILED OUTPUT

Site: **WD PM (NDR/Leeds) - proposed +bunnings + service centre**

New Site  
Roundabout

### OUTPUT TABLE LINKS



#### Roundabouts

Roundabout Basic Parameters  
Roundabout Circulating / Exiting Stream Parameters  
Roundabout Gap Acceptance Parameters  
Roundabout Flow Rates



#### Movements

Intersection Negotiation Data  
Movement Capacity and Performance Parameters  
Fuel Consumption, Emissions and Cost



#### Lanes

Lane Performance and Capacity Information  
Lane Delays  
Lane Queues  
Lane Queue Percentiles  
Lane Stops



#### Flow Rates

Origin-Destination Flow Rates (Total)  
Origin-Destination Flow Rates by Movement Class  
Lane Flow Rates



#### Other

Model Settings Summary  
Diagnostics

### Roundabouts

#### Roundabout Basic Parameters

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

Central Island Diam m	Circ Width m	Insc Diam. m	Entry Radius m	Entry Angle deg	Circ Lanes	Entry Lanes	Av.Entry Lane Width m
South: Leeds Parade south							
36.0	10.0	56.0	20.0	30.0	2	2	3.70
SouthEast: Northern Distributor east							
36.0	10.0	56.0	20.0	30.0	2	3	3.70
NorthEast: Hanrahan Pl							
36.0	10.0	56.0	20.0	30.0	2	1	3.70
North: Leeds Parade north							
36.0	10.0	56.0	20.0	30.0	2	2	3.70
NorthWest: Northern Distributor west							
36.0	10.0	56.0	20.0	30.0	2	2	3.70
Roundabout Capacity Model: SIDRA Standard							

[Go to Table Links \(Top\)](#)

#### Roundabout Circulating / Exiting Stream Parameters

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

Dest	Turn	Lane No.	Lane Type	Opng Flow veh/h	HVE pcu/ veh	Adj. Flow pcu/h	%Near Lane Only	%Exit Flow Incl.	Cap. Const. Effect	O-D Factor	Aver Speed km/h	In-Bunch Headway sec	Prop. Bunched
South: Leeds Parade south													

NW	L1	1	Dominant	585	1.03	605	0.0	0.0	N	0.947	34.4	1.16	0.349
N	T1	1	Dominant	585	1.03	605	0.0	0.0	N	0.947	34.4	1.16	0.349
N	T1	2	Subdominant	585	1.03	605	0.0	0.0	N	0.947	34.4	1.16	0.349
NE	R1	2	Subdominant	585	1.03	605	0.0	0.0	N	0.947	34.4	1.16	0.349
SE	R3	2	Subdominant	585	1.03	605	0.0	0.0	N	0.947	34.4	1.16	0.349
-----													
SouthEast: Northern Distributor east													
S	L3	1	Excl. Slip	348	1.03	360	0.0	0.0	N	0.975	35.0	1.14	0.220
NW	T1	2	Dominant	531	1.04	553	0.0	0.0	N	0.950	32.3	1.36	0.368
NW	T1	3	Subdominant	531	1.04	553	0.0	0.0	N	0.950	32.3	1.36	0.368
N	R1	3	Subdominant	531	1.04	553	0.0	0.0	N	0.950	32.3	1.36	0.368
NE	R2	3	Subdominant	531	1.04	553	0.0	0.0	N	0.950	32.3	1.36	0.368
-----													
NorthEast: Hanrahan Pl													
SE	L2	1	Dominant	848	1.02	869	0.0	0.0	N	0.937	35.4	1.16	0.460
S	L1	1	Dominant	848	1.02	869	0.0	0.0	N	0.937	35.4	1.16	0.460
NW	R2	1	Dominant	848	1.02	869	0.0	0.0	N	0.937	35.4	1.16	0.460
N	R3	1	Dominant	848	1.02	869	0.0	0.0	N	0.937	35.4	1.16	0.460
-----													
North: Leeds Parade north													
NE	L3	1	Dominant	587	1.04	611	0.0	0.0	N	0.936	33.5	1.10	0.335
SE	L1	1	Dominant	587	1.04	611	0.0	0.0	N	0.936	33.5	1.10	0.335
S	T1	1	Dominant	587	1.04	611	0.0	0.0	N	0.936	33.5	1.10	0.335
S	T1	2	Subdominant	587	1.04	611	0.0	0.0	N	0.936	33.5	1.10	0.335
NW	R3	2	Subdominant	587	1.04	611	0.0	0.0	N	0.936	33.5	1.10	0.335
-----													
NorthWest: Northern Distributor west													
N	L3	1	Dominant	470	1.03	486	0.0	0.0	N	0.948	35.3	1.54	0.365
NE	L2	1	Dominant	470	1.03	486	0.0	0.0	N	0.948	35.3	1.54	0.365
SE	T1	1	Dominant	470	1.03	486	0.0	0.0	N	0.948	35.3	1.54	0.365
SE	T1	2	Subdominant	470	1.03	486	0.0	0.0	N	0.948	35.3	1.54	0.365
S	R1	2	Subdominant	470	1.03	486	0.0	0.0	N	0.948	35.3	1.54	0.365
-----													
Roundabout Capacity Model: SIDRA Standard													

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## Roundabout Gap Acceptance Parameters

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

Dest	Turn	Lane No.	Lane Type	In-Bunch Headway sec	Priority Sharing	Critical Gap		Follow-up Headway sec
						Headway sec	Dist m	
-----								
South: Leeds Parade south								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
NW	L1	1	Dominant	1.16	N	3.38	32.3	2.13
N	T1	1	Dominant	1.16	N	3.38	32.3	2.13
N	T1	2	Subdominant	1.16	N	3.89	37.1	2.45
NE	R1	2	Subdominant	1.16	N	4.15	39.6	2.61
SE	R3	2	Subdominant	1.16	N	3.89	37.1	2.45
-----								
SouthEast: Northern Distributor east								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
S	L3	1	Excl. Slip	1.14	N	3.06	29.7	1.84
NW	T1	2	Dominant	1.36	Y	2.84	25.4	1.77
NW	T1	3	Subdominant	1.36	Y	3.56	31.9	2.22
N	R1	3	Subdominant	1.36	Y	3.56	31.9	2.22
NE	R2	3	Subdominant	1.36	Y	3.56	31.9	2.22
-----								
NorthEast: Hanrahan Pl								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
SE	L2	1	Dominant	1.16	N	5.38	52.9	3.52
S	L1	1	Dominant	1.16	N	5.38	52.9	3.52
NW	R2	1	Dominant	1.16	N	5.38	52.9	3.52
N	R3	1	Dominant	1.16	N	5.38	52.9	3.52
-----								
North: Leeds Parade north								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
NE	L3	1	Dominant	1.10	N	3.41	31.8	2.15
SE	L1	1	Dominant	1.10	N	3.38	31.4	2.13
S	T1	1	Dominant	1.10	N	3.38	31.4	2.13
S	T1	2	Subdominant	1.10	N	3.87	36.1	2.44
NW	R3	2	Subdominant	1.10	N	3.87	36.1	2.44
-----								
NorthWest: Northern Distributor west								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
N	L3	1	Dominant	1.54	Y	3.52	34.5	2.17
NE	L2	1	Dominant	1.54	Y	3.83	37.6	2.37
SE	T1	1	Dominant	1.54	Y	3.52	34.5	2.17



SE	T1	2 Subdominant	1.54	Y	3.98	39.0	2.46
S	R1	2 Subdominant	1.54	Y	3.98	39.0	2.46

Roundabout Capacity Model: SIDRA Standard  
 Priority sharing means Follow-up Headway plus Intra-bunch Headway  
 is larger than the Critical Gap.

Dist (Distance): Spacing, i.e. distance between the front ends of two  
 successive vehicles across all lanes in the circulating  
 or exiting stream

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## Roundabout Flow Rates

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
 Roundabout

### CIRCULATING LANE FLOW RATES

Lane No.	Circulating Flow Rates		
	veh/h	pcu/h	Percent
South: Leeds Parade south			
1	230	236	39.0%
2	355	370	61.0%
Total	585	606	
SouthEast: Northern Distributor east			
1	133	137	24.7%
2	398	417	75.3%
Total	531	554	
NorthEast: Hanrahan Pl			
1	336	344	39.6%
2	512	525	60.4%
Total	848	869	
North: Leeds Parade north			
1	328	345	56.5%
2	259	266	43.5%
Total	587	611	
NorthWest: Northern Distributor west			
1	61	62	12.8%
2	409	424	87.2%
Total	470	486	

### APPROACH LANE FLOW RATES

Lane No.	Approach Flows (veh/h)		
	Out	To Downst	Total
South: Leeds Parade south			
1	299	61	360
2	0	292	292
Total	299	353	652
SouthEast: Northern Distributor east			
1	57	0	57
2	0	230	230
3	0	172	172
Total	57	402	459
NorthEast: Hanrahan Pl			
1	5	19	24
Total	5	19	24
North: Leeds Parade north			
1	49	209	258
2	0	215	215
Total	49	424	473
NorthWest: Northern Distributor west			
1	111	186	297
2	0	259	259
Total	111	445	556

### EXITING LANE FLOW RATES

Lane	Exiting Flow Rates
------	--------------------

No.	veh/h	pcu/h	Percent
SouthEast: Northern Distributor east			
1	140	147	40.9%
2	208	213	59.1%
Total	348	360	

[Go to Table Links \(Top\)](#)

## Movements

### Intersection Negotiation Data

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

#### INTERSECTION NEGOTIATION DATA

From Approach	To Exit	Turn	Negn Radius m	Negn Speed km/h	Negn Dist. m	Appr. Dist. m	Downstream m	Distance User Spec?
South: Leeds Parade south								
	NorthWest	L1	68.6	41.9	39.9	500	177	No
	North	T1	68.6	41.9	53.2	500	197	No
	NorthEast	R1	22.0	27.2	69.1	500	303	No
	SouthEast	R3	22.0	27.2	103.7	500	234	No
SouthEast: Northern Distributor east								
	South	L3	33.0	31.7	11.0	500	181	No
	NorthWest	T1	68.6	41.9	53.2	500	213	No
	North	R1	22.0	27.2	69.1	500	216	No
	NorthEast	R2	22.0	27.2	86.4	500	232	No
NorthEast: Hanrahan Pl								
	SouthEast	L2	46.6	36.2	21.6	500	1088	No
	South	L1	68.6	41.9	39.9	500	1209	No
	NorthWest	R2	22.0	27.2	86.4	500	972	No
	North	R3	22.0	27.2	103.7	500	989	No
North: Leeds Parade north								
	NorthEast	L3	30.6	30.8	11.0	500	168	No
	SouthEast	L1	68.6	41.9	39.9	500	185	No
	South	T1	68.6	41.9	53.2	500	217	No
	NorthWest	R3	22.0	27.2	103.7	500	254	No
NorthWest: Northern Distributor west								
	North	L3	30.6	30.8	11.0	500	165	No
	NorthEast	L2	42.9	35.0	21.7	500	335	No
	SouthEast	T1	68.6	41.9	53.2	500	211	No
	South	R1	22.0	27.2	69.1	500	214	No

Maximum Negotiation (Design) Speed = 50.0 km/h

Downstream distance is distance travelled from the stopline until exit cruise speed is reached (includes negotiation distance). Acceleration distance is weighted for light and heavy vehicles. The same distance applies for both stopped and unstopped vehicles.

#### MOVEMENT SPEEDS AND GEOMETRIC DELAY

						Queue Move-up				
Mov	Turn	App. Speeds		Exit Speeds				Av. Section Spd		Geom
ID		Cruise	Negn	Negn	Cruise	1st Grn	2nd Grn	Running	Overall	Delay sec
South: Leeds Parade south										
1a	L1	60.0	41.9	41.9	60.0	20.8		49.6	49.6	4.7
2	T1	60.0	41.9	41.9	60.0	20.0		48.3	48.1	4.7
3a	R1	60.0	27.2	27.2	60.0	19.7		47.9	47.7	14.7
3b	R3	60.0	27.2	27.2	60.0	19.7		47.9	47.7	14.2
SouthEast: Northern Distributor east										
21b	L3	60.0	31.7	31.7	60.0	27.1		49.3	49.3	7.3
22	T1	60.0	41.9	41.9	60.0	23.2		49.4	49.4	4.7
23a	R1	60.0	27.2	27.2	60.0	21.4		46.7	46.7	11.7
23	R2	60.0	27.2	27.2	60.0	21.4		46.7	46.7	12.9
NorthEast: Hanrahan Pl										
24	L2	60.0	36.2	36.2	60.0	20.1		42.2	42.2	23.4
24a	L1	60.0	41.9	41.9	60.0	20.1		42.2	42.2	18.2

26	R2	60.0	27.2	27.2	60.0	20.1	42.2	42.2	32.2
26b	R3	60.0	27.2	27.2	60.0	20.1	42.2	42.2	33.4
-----									
North: Leeds Parade north									
7b	L3	60.0	30.8	30.8	60.0	20.7	49.4	49.4	8.0
7a	L1	60.0	41.9	41.9	60.0	20.7	49.4	49.4	4.7
8	T1	60.0	41.9	41.9	60.0	20.4	48.1	48.1	4.7
9b	R3	60.0	27.2	27.2	60.0	19.5	45.0	45.0	14.2
-----									
NorthWest: Northern Distributor west									
27b	L3	60.0	30.8	30.8	60.0	23.0	48.8	48.8	7.5
27	L2	60.0	35.0	35.0	60.0	23.0	48.8	48.8	9.9
28	T1	60.0	41.9	41.9	60.0	22.4	47.9	47.9	4.7
29a	R1	60.0	27.2	27.2	60.0	21.7	46.9	46.9	11.7
-----									
"Running Speed" is the average speed excluding stopped periods.									

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## Movement Capacity and Performance Parameters

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

### MOVEMENT CAPACITY PARAMETERS

Mov ID	Turn	Mov Cl.	Arv Flow veh/h	Opng Flow veh/h	Movement Adjust. Flow pcu/h	Total Cap. veh/h	Prac. Deg. Satn xp	Prac. Spare Cap. %	Deg. Satn x	
-----										
South: Leeds Parade south										
1a	L1	#	299	585	605	907	0.85	158	0.330*	
2	T1	#	259	1170	1211	785	0.85	158	0.330*	
3a	R1	#	56	585	605	170	0.85	158	0.330*	
3b	R3	#	38	585	605	115	0.85	158	0.330*	
-----										
SouthEast: Northern Distributor east										
21b	L3	#	57	348	360	1541	0.85	2198	0.037	
22	T1	#	286	1062	1107	1758	0.85	422	0.163	
23a	R1	#	68	531	553	418	0.85	422	0.163	
23	R2	#	48	531	553	295	0.85	422	0.163	
-----										
NorthEast: Hanrahan Pl										
24	L2	LV	0	0	0	0.000	0.000	0	0.85	**** 0.000
24	L2	HV	5	88	0	0.000	0.000	88	0.85	1403 0.057
24a	L1	LV	0	0	0	0.000	0.000	0	0.85	**** 0.000
24a	L1	HV	7	124	0	0.000	0.000	124	0.85	1403 0.057
26	R2	LV	0	0	0	0.000	0.000	0	0.85	**** 0.000
26	R2	HV	11	194	0	0.000	0.000	194	0.85	1403 0.057
26b	R3	LV	0	0	0	0.000	0.000	0	0.85	**** 0.000
26b	R3	HV	1	18	0	0.000	0.000	18	0.85	1403 0.057
-----										
North: Leeds Parade north										
7b	L3	#	49	587	611	204	0.85	254	0.240	
7a	L1	#	76	587	611	317	0.85	254	0.240	
8	T1	#	177	1174	1222	738	0.85	254	0.240	
9b	R3	#	171	587	611	713	0.85	254	0.240	
-----										
NorthWest: Northern Distributor west										
27b	L3	#	111	470	486	410	0.85	214	0.271	
27	L2	#	59	470	486	218	0.85	214	0.271	
28	T1	#	222	940	972	820	0.85	214	0.271	
29a	R1	#	164	470	486	606	0.85	214	0.271	
-----										
* Maximum degree of saturation										
# Combined Movement Capacity parameters are shown for all Movement Classes.										

### MOVEMENT PERFORMANCE

Mov ID	Turn	Total Delay (veh-h/h)	Total Delay (pers-h/h)	Aver. Delay (sec)	Eff. Stop Rate	Total Stops	Perf. Index	Tot.Trav. Distance (veh-km/h)	Tot.Trav. Time (veh-h/h)	Aver. Speed (km/h)
-----										
South: Leeds Parade south										
1a	L1	0.03	0.03	6.6	1.08	324.3	11.92	378.1	7.6	49.6
2	T1	0.03	0.03	9.5	1.35	350.4	11.20	335.9	7.0	48.1
3a	R1	0.05	0.06	10.4	1.43	80.3	4.81	73.2	1.5	47.7
3b	R3	0.01	0.01	10.4	1.43	54.5	4.09	49.7	1.0	47.7
-----										
SouthEast: Northern Distributor east										
21b	L3	0.01	0.01	8.0	1.10	62.9	2.07	72.5	1.5	49.3
22	T1	0.02	0.03	7.0	1.05	299.2	10.05	366.8	7.4	49.4
23a	R1	0.01	0.02	11.5	1.43	97.1	3.82	91.4	2.0	46.7
23	R2	0.01	0.01	11.5	1.43	68.5	3.10	64.5	1.4	46.7

-----										
NorthEast: Hanrahan Pl										
24	L2	0.04	0.05	32.2	1.50	7.5	0.55	15.4	0.4	42.2
24a	L1	0.05	0.06	32.2	1.50	10.5	0.68	21.6	0.5	42.2
26	R2	0.12	0.14	32.2	1.50	16.5	1.06	34.0	0.8	42.2
26b	R3	0.01	0.01	32.2	1.50	1.5	0.25	3.1	0.1	42.2
-----										
North: Leeds Parade north										
7b	L3	0.01	0.01	7.1	1.13	55.6	3.48	62.1	1.3	49.4
7a	L1	0.01	0.01	7.1	1.13	86.2	4.23	96.3	2.0	49.4
8	T1	0.02	0.02	8.9	1.25	221.3	7.57	230.2	4.8	48.1
9b	R3	0.04	0.05	14.5	1.60	274.3	8.88	239.6	5.3	45.0
-----										
NorthWest: Northern Distributor west										
27b	L3	0.01	0.02	8.6	1.24	137.9	6.05	141.0	2.9	48.8
27	L2	0.04	0.05	8.6	1.24	73.3	4.39	74.9	1.5	48.8
28	T1	0.02	0.02	9.7	1.33	296.2	9.58	287.7	6.0	47.9
29a	R1	0.03	0.04	11.3	1.46	238.9	8.50	218.1	4.7	46.9
-----										

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## Fuel Consumption, Emissions and Cost

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

### FUEL CONSUMPTION, EMISSIONS AND COST (TOTAL)

Mov ID	Turn	Cost Total \$/h	Fuel Total L/h	CO2 Total kg/h	CO Total kg/h	HC Total kg/h	NOX Total kg/h
-----							
South: Leeds Parade south							
1a	L1	143.24	13.0	30.8	0.09	0.010	0.089
2	T1	137.57	13.5	32.1	0.09	0.010	0.110
3a	R1	30.64	3.1	7.3	0.02	0.002	0.026
3b	R3	20.79	2.1	5.0	0.01	0.001	0.018
		332.24	31.7	75.1	0.21	0.024	0.242
-----							
SouthEast: Northern Distributor east							
21b	L3	27.57	2.5	5.9	0.02	0.002	0.017
22	T1	138.80	12.6	29.7	0.09	0.010	0.085
23a	R1	36.00	3.1	7.4	0.02	0.003	0.021
23	R2	25.41	2.2	5.3	0.02	0.002	0.015
		227.78	20.4	48.3	0.14	0.016	0.137
-----							
NorthEast: Hanrahan Pl							
24	L2	9.33	2.0	5.2	0.01	0.001	0.030
24a	L1	13.06	2.7	7.2	0.01	0.001	0.042
26	R2	20.52	4.3	11.4	0.02	0.001	0.066
26b	R3	1.87	0.4	1.0	0.00	0.000	0.006
		44.78	9.4	24.8	0.03	0.003	0.143
-----							
North: Leeds Parade north							
7b	L3	23.86	2.2	5.2	0.01	0.002	0.016
7a	L1	37.01	3.4	8.1	0.02	0.003	0.024
8	T1	89.65	8.1	19.2	0.05	0.006	0.056
9b	R3	96.80	8.3	19.5	0.06	0.007	0.053
		247.32	22.0	52.1	0.15	0.018	0.149
-----							
NorthWest: Northern Distributor west							
27b	L3	59.73	6.3	15.0	0.04	0.004	0.057
27	L2	31.75	3.4	8.0	0.02	0.002	0.030
28	T1	118.06	11.6	27.5	0.07	0.008	0.094
29a	R1	85.83	7.5	17.8	0.05	0.006	0.050
		295.37	28.8	68.3	0.18	0.021	0.230
-----							
INTERSECTION:		1136.12	112.3	268.4	0.71	0.082	0.901
-----							

### FUEL CONSUMPTION, EMISSIONS AND COST (RATE)

Mov ID	Turn	Cost Rate \$/km	Fuel Rate L/100km	CO2 Rate g/km	CO Rate g/km	HC Rate g/km	NOX Rate g/km
-----							
South: Leeds Parade south							
1a	L1	0.38	3.4	81.4	0.23	0.027	0.236
2	T1	0.41	4.0	95.5	0.25	0.029	0.326

3a	R1	0.42	4.2	99.7	0.26	0.030	0.353
3b	R3	0.42	4.2	99.7	0.26	0.030	0.353
		0.40	3.8	89.8	0.25	0.028	0.289
-----							
SouthEast: Northern Distributor east							
21b	L3	0.38	3.5	81.6	0.23	0.027	0.236
22	T1	0.38	3.4	80.9	0.23	0.027	0.232
23a	R1	0.39	3.4	81.4	0.24	0.029	0.225
23	R2	0.39	3.4	81.4	0.24	0.029	0.225
		0.38	3.4	81.1	0.23	0.028	0.231
-----							
NorthEast: Hanrahan Pl							
24	L2	0.60	12.7	335.0	0.45	0.036	1.930
24a	L1	0.60	12.7	335.0	0.45	0.036	1.930
26	R2	0.60	12.7	335.0	0.45	0.036	1.930
26b	R3	0.60	12.7	335.0	0.45	0.036	1.930
		0.60	12.7	335.0	0.45	0.036	1.930
-----							
North: Leeds Parade north							
7b	L3	0.38	3.6	84.1	0.24	0.028	0.253
7a	L1	0.38	3.6	84.1	0.24	0.028	0.253
8	T1	0.39	3.5	83.4	0.24	0.028	0.244
9b	R3	0.40	3.5	81.6	0.24	0.029	0.220
		0.39	3.5	82.9	0.24	0.028	0.237
-----							
NorthWest: Northern Distributor west							
27b	L3	0.42	4.5	106.6	0.27	0.030	0.403
27	L2	0.42	4.5	106.6	0.27	0.030	0.403
28	T1	0.41	4.0	95.6	0.25	0.029	0.326
29a	R1	0.39	3.5	81.6	0.24	0.028	0.227
		0.41	4.0	94.7	0.25	0.029	0.319
-----							
INTERSECTION:		0.66	6.6	156.6	0.42	0.048	0.526
-----							

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## Lanes

### Lane Performance and Capacity Information

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

#### LANE PERFORMANCE

						Q u e u e		
Lane	Flow	Cap	Deg.	Aver.	Eff.	95% Back		Lane
No.	veh/h	veh/h	Satn x	Delay sec	Stop Rate	veh	m	Length m
South: Leeds Parade south								
1	360	1091	0.330	6.6	0.54	1.7	12.4	60.0
2	292	887	0.330	10.4	0.72	1.6	12.1	500.0
SouthEast: Northern Distributor east								
1	57	1541	0.037	8.0	0.55	0.2	1.2	20.0
2	230	1414	0.163	5.9	0.48	0.8	6.1	120.0
3	172	1057	0.163	11.5	0.71	0.8	5.7	500.0
NorthEast: Hanrahan Pl								
1	24	424	0.057	32.2	0.75	0.2	2.7	190.0
North: Leeds Parade north								
1	258	1076	0.240	7.1	0.57	1.2	8.5	100.0
2	215	895	0.240	14.5	0.80	1.1	8.2	500.0
NorthWest: Northern Distributor west								
1	297	1097	0.271	8.6	0.62	1.5	11.0	60.0
2	259	958	0.271	11.3	0.73	1.4	10.3	500.0

#### LANE FLOW AND CAPACITY INFORMATION

Lane No.	Total Arv Flow (veh/h)	Min Cap veh/h	Tot Cap veh/h	Deg. Satn x	Lane Util %
----------	------------------------------	---------------------	---------------------	-------------------	-------------------



South: Leeds Parade south  
 1 360 150 1091 0.330 100  
 2 292 150 887 0.330 100

SouthEast: Northern Distributor east  
 1 57 57 1541 0.037 100  
 2 230 150 1414 0.163 100  
 3 172 150 1057 0.163 100

NorthEast: Hanrahan Pl  
 1 24 24 424 0.057 100

North: Leeds Parade north  
 1 258 150 1076 0.240 100  
 2 215 150 895 0.240 100

NorthWest: Northern Distributor west  
 1 297 150 1097 0.271 100  
 2 259 150 958 0.271 100

The capacity value for priority and continuous movements is obtained by adjusting the basic saturation flow for heavy vehicle and turning vehicle effects. Saturation flow scale applies if specified.

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## Lane Delays

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
 Roundabout

### LANE DELAYS

Lane No.	Deg. Satn x	Prog. Factor	Stop-line Delay			Delay (seconds/veh)					
			1st d1	2nd d2	Total dSL	Acc. Dec. dn	Queuing Total dq	MvUp dqm	Stopd (Idle) di	Geom dig	Control dic
South: Leeds Parade south											
1	0.330	1.000	1.9	0.0	1.9	4.0	0.0	0.0	0.0	4.7	6.6
2	0.330	1.000	2.5	0.0	2.5	3.5	0.2	0.0	0.2	7.9	10.4
SouthEast: Northern Distributor east											
1	0.037	1.000	0.6	0.0	0.6	2.1	0.0	0.0	0.0	7.3	8.0
2	0.163	1.000	1.2	0.0	1.2	3.5	0.0	0.0	0.0	4.7	5.9
3	0.163	1.000	1.7	0.0	1.7	2.6	0.0	0.0	0.0	9.8	11.5
NorthEast: Hanrahan Pl											
1	0.057	1.000	5.9	0.0	5.9	12.3	0.0	0.0	0.0	26.3	32.2
North: Leeds Parade north											
1	0.240	1.000	1.7	0.0	1.7	3.7	0.0	0.0	0.0	5.3	7.1
2	0.240	1.000	2.2	0.0	2.2	2.7	0.0	0.0	0.0	12.3	14.5
NorthWest: Northern Distributor west											
1	0.271	1.000	1.8	0.0	1.8	3.5	0.0	0.0	0.0	6.8	8.6
2	0.271	1.000	2.1	0.0	2.1	2.9	0.0	0.0	0.0	9.2	11.3

SIDRA Standard Delay Model is used. Control Delay is the sum of Stop-line Delay and Geometric Delay.

dSL: Stop-line delay (=d1+d2)

dn: Average stop-start delay for all vehicles queued and unqueued

dq: Queuing delay (the part of the stop-line delay that includes stopped delay and queue move-up delay)

dqm: Queue move-up delay

di: Stopped delay (stopped (idling) time at near-zero speed)

dig: Geometric delay

dic: Control delay

### LANE DELAY PERCENTILES

Lane No.	Deg. Satn	Percentile Delay						
	x	50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade south								
1	0.330	6.6	6.9	7.5	7.8	8.1	8.4	8.6
2	0.330	10.4	10.8	11.6	12.0	12.4	12.8	13.0
SouthEast: Northern Distributor east								
1	0.037	8.0	8.1	8.3	8.4	8.5	8.6	8.6
2	0.163	5.9	6.1	6.5	6.6	6.8	7.0	7.1
3	0.163	11.5	11.8	12.4	12.6	12.9	13.2	13.3

NorthEast: Hanrahan Pl								
1	0.057	32.2	33.3	35.0	35.9	36.9	37.7	38.3
-----								
North: Leeds Parade north								
1	0.240	7.1	7.4	7.9	8.2	8.5	8.7	8.9
2	0.240	14.5	14.9	15.6	15.9	16.3	16.6	16.8
-----								
NorthWest: Northern Distributor west								
1	0.271	8.6	8.9	9.4	9.7	10.0	10.2	10.4
2	0.271	11.3	11.7	12.3	12.6	13.0	13.2	13.5
-----								

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## Lane Queues

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

### LANE QUEUES (VEHICLES)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (veh)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Queue	
	x		No	Nb1	Nb2	Nb	95%				Nc	95%
South: Leeds Parade south												
1	0.330	1.000	0.0	0.7	0.0	0.7	1.7	0.08	0.0	100.0	0.2	0.3
2	0.330	1.000	0.0	0.7	0.0	0.7	1.6	0.01	0.0	100.0	0.2	0.4
SouthEast: Northern Distributor east												
1	0.037	1.000	0.0	0.1	0.0	0.1	0.2	0.02	0.0	100.0	0.0	0.0
2	0.163	1.000	0.0	0.3	0.0	0.3	0.8	0.02	0.0	100.0	0.1	0.1
3	0.163	1.000	0.0	0.3	0.0	0.3	0.8	0.00	0.0	100.0	0.1	0.2
NorthEast: Hanrahan Pl												
1	0.057	1.000	0.0	0.1	0.0	0.1	0.2	0.01	0.0	100.0	0.0	0.1
North: Leeds Parade north												
1	0.240	1.000	0.0	0.5	0.0	0.5	1.2	0.03	0.0	100.0	0.1	0.2
2	0.240	1.000	0.0	0.5	0.0	0.5	1.1	0.01	0.0	100.0	0.1	0.2
NorthWest: Northern Distributor west												
1	0.271	1.000	0.0	0.6	0.0	0.6	1.5	0.07	0.0	100.0	0.1	0.3
2	0.271	1.000	0.0	0.6	0.0	0.6	1.4	0.01	0.0	100.0	0.2	0.3

### LANE QUEUES (DISTANCE)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (m)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Nc	Queue 95%
	x			Nb1	Nb2	Nb	95%					
South: Leeds Parade south												
1	0.330	1.000	0.0	5.0	0.0	5.0	12.4	0.08	0.0	100.0	1.4	2.5
2	0.330	1.000	0.0	4.9	0.0	4.9	12.1	0.01	0.0	100.0	1.5	2.8
SouthEast: Northern Distributor east												
1	0.037	1.000	0.0	0.5	0.0	0.5	1.2	0.02	0.0	100.0	0.1	0.1
2	0.163	1.000	0.0	2.4	0.0	2.4	6.1	0.02	0.0	100.0	0.6	1.0
3	0.163	1.000	0.0	2.3	0.0	2.3	5.7	0.00	0.0	100.0	0.6	1.1
NorthEast: Hanrahan Pl												
1	0.057	1.000	0.0	1.1	0.0	1.1	2.7	0.01	0.0	100.0	0.5	0.9
North: Leeds Parade north												
1	0.240	1.000	0.0	3.4	0.0	3.4	8.5	0.03	0.0	100.0	0.9	1.7
2	0.240	1.000	0.0	3.3	0.0	3.3	8.2	0.01	0.0	100.0	1.0	1.8
NorthWest: Northern Distributor west												
1	0.271	1.000	0.0	4.4	0.0	4.4	11.0	0.07	0.0	100.0	1.1	2.0
2	0.271	1.000	0.0	4.2	0.0	4.2	10.3	0.01	0.0	100.0	1.1	2.0

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## Lane Queue Percentiles

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

## LANE QUEUE PERCENTILES (VEHICLES)

Lane No.	Deg. Satn	Percentile Back of Queue (veh)						
	x	50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade south								
1	0.330	0.7	0.9	1.2	1.4	1.7	1.9	2.0
2	0.330	0.7	0.8	1.2	1.4	1.6	1.8	1.9
SouthEast: Northern Distributor east								
1	0.037	0.1	0.1	0.1	0.1	0.2	0.2	0.2
2	0.163	0.3	0.4	0.6	0.7	0.8	0.9	1.0
3	0.163	0.3	0.4	0.6	0.7	0.8	0.9	0.9
NorthEast: Hanrahan Pl								
1	0.057	0.1	0.1	0.2	0.2	0.2	0.2	0.2
North: Leeds Parade north								
1	0.240	0.5	0.6	0.9	1.0	1.2	1.3	1.4
2	0.240	0.4	0.6	0.8	1.0	1.1	1.2	1.3
NorthWest: Northern Distributor west								
1	0.271	0.6	0.8	1.1	1.2	1.5	1.6	1.7
2	0.271	0.6	0.7	1.0	1.2	1.4	1.6	1.7

## LANE QUEUE PERCENTILES (DISTANCE)

Lane No.	Deg. Satn	Percentile Back of Queue (metres)						
	x	50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade south								
1	0.330	5.0	6.4	9.1	10.5	12.4	13.7	14.7
2	0.330	4.9	6.3	8.9	10.3	12.1	13.4	14.4
SouthEast: Northern Distributor east								
1	0.037	0.5	0.6	0.9	1.0	1.2	1.3	1.4
2	0.163	2.4	3.2	4.5	5.2	6.1	6.8	7.3
3	0.163	2.3	2.9	4.2	4.8	5.7	6.3	6.8
NorthEast: Hanrahan Pl								
1	0.057	1.1	1.4	2.0	2.3	2.7	3.0	3.2
North: Leeds Parade north								
1	0.240	3.4	4.4	6.3	7.2	8.5	9.5	10.2
2	0.240	3.3	4.3	6.0	6.9	8.2	9.1	9.7
NorthWest: Northern Distributor west								
1	0.271	4.4	5.7	8.1	9.4	11.0	12.2	13.1
2	0.271	4.2	5.4	7.6	8.8	10.3	11.5	12.3

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## Lane Stops

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

							Queue	Total		
Lane	Deg.	Prog.	-- Effective		Stop	Rate --	Total	Move-up	Queue	Prop.
No.	Satn	Factor	he1	he2	Geom.	Overall	Stops	Rate	Move-ups	Queued
	x				hig	h	H	hqm	Hqm	pq
South: Leeds Parade south										
1	0.330	1.000	0.39	0.00	0.15	0.54	195.1	0.00	0.0	0.58
2	0.330	1.000	0.50	0.00	0.22	0.72	209.7	0.00	0.0	0.60
SouthEast: Northern Distributor east										
1	0.037	1.000	0.21	0.00	0.34	0.55	31.5	0.00	0.0	0.36
2	0.163	1.000	0.29	0.00	0.18	0.48	109.6	0.00	0.0	0.50
3	0.163	1.000	0.41	0.00	0.30	0.71	122.8	0.00	0.0	0.53
NorthEast: Hanrahan Pl										
1	0.057	1.000	0.50	0.00	0.25	0.75	17.9	0.00	0.0	0.62
North: Leeds Parade north										
1	0.240	1.000	0.38	0.00	0.19	0.57	146.5	0.00	0.0	0.56
2	0.240	1.000	0.51	0.00	0.29	0.80	172.2	0.00	0.0	0.58
NorthWest: Northern Distributor west										

1	0.271	1.000	0.41	0.00	0.21	0.62	184.3	0.00	0.0	0.56
2	0.271	1.000	0.46	0.00	0.27	0.73	188.9	0.00	0.0	0.57

hig is the average value for all movements in a shared lane  
hqm is average queue move-up rate for all vehicles queued and unqueued

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## Flow Rates

### Origin-Destination Flow Rates (Total)

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

#### TOTAL FLOW RATES (ALL MOVEMENT CLASSES)

From SOUTH To:	NW	N	NE	SE	
Turn:	L1	T1	R1	R3	TOT
Flow Rate	299.0	259.0	56.0	38.0	652.0
%HV (all designations)	5.0	5.0	19.0	5.0	6.2
From SOUTHEAST To:	S	NW	N	NE	
Turn:	L3	T1	R1	R2	TOT
Flow Rate	57.0	286.0	68.0	48.0	459.0
%HV (all designations)	5.0	5.0	5.0	5.0	5.0
From NORTHEAST To:	SE	S	NW	N	
Turn:	L2	L1	R2	R3	TOT
Flow Rate	5.0	7.0	11.0	1.0	24.0
%HV (all designations)	100.0	100.0	100.0	100.0	100.0
From NORTH To:	NE	SE	S	NW	
Turn:	L3	L1	T1	R3	TOT
Flow Rate	49.0	76.0	177.0	171.0	473.0
%HV (all designations)	7.0	5.0	5.0	5.0	5.2
From NORTHWEST To:	N	NE	SE	S	
Turn:	L3	L2	T1	R1	TOT
Flow Rate	111.0	59.0	222.0	164.0	556.0
%HV (all designations)	5.0	23.0	5.0	5.0	6.9

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### Origin-Destination Flow Rates by Movement Class

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

#### FLOW RATES FOR Light Vehicles

From SOUTH To:	NW	N	NE	SE	
Turn:	L1	T1	R1	R3	TOT
Flow Rate - Veh	284.0	246.1	45.4	36.1	611.6
Mov Class %	95.0	95.0	81.0	95.0	93.8
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
From SOUTHEAST To:	S	NW	N	NE	
Turn:	L3	T1	R1	R2	TOT
Flow Rate - Veh	54.2	271.7	64.6	45.6	436.1
Mov Class %	95.0	95.0	95.0	95.0	95.0
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
From NORTHEAST To:	SE	S	NW	N	
Turn:	L2	L1	R2	R3	TOT
Flow Rate - Veh	0.0	0.0	0.0	0.0	0.0
Mov Class %	0.0	0.0	0.0	0.0	0.0
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
From NORTH To:	NE	SE	S	NW	
Turn:	L3	L1	T1	R3	TOT

-----					
Flow Rate - Veh	45.6	72.2	168.1	162.4	448.4
Mov Class %	93.0	95.0	95.0	95.0	94.8
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTHWEST To:	N	NE	SE	S	
Turn:	L3	L2	T1	R1	TOT
-----					
Flow Rate - Veh	105.4	45.4	210.9	155.8	517.6
Mov Class %	95.0	77.0	95.0	95.0	93.1
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
FLOW RATES FOR Heavy Vehicles					
-----					
From SOUTH To:	NW	N	NE	SE	
Turn:	L1	T1	R1	R3	TOT
-----					
Flow Rate - Veh	14.9	12.9	10.6	1.9	40.4
Mov Class %	5.0	5.0	19.0	5.0	6.2
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From SOUTHEAST To:	S	NW	N	NE	
Turn:	L3	T1	R1	R2	TOT
-----					
Flow Rate - Veh	2.8	14.3	3.4	2.4	23.0
Mov Class %	5.0	5.0	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTHEAST To:	SE	S	NW	N	
Turn:	L2	L1	R2	R3	TOT
-----					
Flow Rate - Veh	5.0	7.0	11.0	1.0	24.0
Mov Class %	100.0	100.0	100.0	100.0	100.0
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTH To:	NE	SE	S	NW	
Turn:	L3	L1	T1	R3	TOT
-----					
Flow Rate - Veh	3.4	3.8	8.9	8.6	24.6
Mov Class %	7.0	5.0	5.0	5.0	5.2
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTHWEST To:	N	NE	SE	S	
Turn:	L3	L2	T1	R1	TOT
-----					
Flow Rate - Veh	5.6	13.6	11.1	8.2	38.4
Mov Class %	5.0	23.0	5.0	5.0	6.9
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					

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### Lane Flow Rates

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

#### LANE FLOW RATES AT STOP LINE

-----					
From SOUTH To:	NW	N	NE	SE	
Turn:	L1	T1	R1	R3	TOT
-----					
Lane 1					
LV	284.0	57.6	*	*	341.7
HV	14.9	3.0	*	*	18.0
Total	299.0	60.6	*	*	359.6
Lane 2					
LV	*	188.4	45.4	36.1	269.9
HV	*	9.9	10.6	1.9	22.5
Total	*	198.4	56.0	38.0	292.4
-----					



Approach	299.0	259.0	56.0	38.0	652.0
From SOUTHEAST To:	S	NW	N	NE	
Turn:	L3	T1	R1	R2	TOT
Lane 1					
LV	54.2	*	*	*	54.2
HV	2.8	*	*	*	2.8
Total	57.0	*	*	*	57.0
Lane 2					
LV	*	218.5	*	*	218.5
HV	*	11.5	*	*	11.5
Total	*	230.1	*	*	230.1
Lane 3					
LV	*	53.2	64.6	45.6	163.4
HV	*	2.8	3.4	2.4	8.6
Total	*	55.9	68.0	48.0	171.9
Approach	57.0	286.0	68.0	48.0	459.0
From NORTHEAST To:	SE	S	NW	N	
Turn:	L2	L1	R2	R3	TOT
Lane 1					
HV	5.0	7.0	11.0	1.0	24.0
Total	5.0	7.0	11.0	1.0	24.0
Approach	5.0	7.0	11.0	1.0	24.0
From NORTH To:	NE	SE	S	NW	
Turn:	L3	L1	T1	R3	TOT
Lane 1					
LV	45.6	72.2	126.6	*	244.4
HV	3.4	3.8	6.7	*	13.9
Total	49.0	76.0	133.3	*	258.3
Lane 2					
LV	*	*	41.5	162.4	204.0
HV	*	*	2.2	8.6	10.7
Total	*	*	43.7	171.0	214.7
Approach	49.0	76.0	177.0	171.0	473.0
From NORTHWEST To:	N	NE	SE	S	
Turn:	L3	L2	T1	R1	TOT
Lane 1					
LV	105.4	45.4	120.4	*	271.3
HV	5.6	13.6	6.3	*	25.5
Total	111.0	59.0	126.7	*	296.7
Lane 2					
LV	*	*	90.5	155.8	246.3
HV	*	*	4.8	8.2	13.0
Total	*	*	95.3	164.0	259.3
Approach	111.0	59.0	222.0	164.0	556.0

\* Movement not allocated to the lane

## EXIT LANE FLOW RATES

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	180.8	16.5
Lane: 2	197.3	10.4
Total	378.1	26.9
Exit: SOUTHEAST		
Lane: 1	192.6	15.1
Lane: 2	126.6	6.7
Total	319.2	21.8
Exit: NORTHEAST		
Lane: 1	182.0	30.0
Total	182.0	30.0
Exit: NORTH		
Lane: 1	163.1	8.6
Lane: 2	253.0	14.3
Total	416.1	22.9
Exit: NORTHWEST		
Lane: 1	502.6	26.5
Lane: 2	215.6	22.3
Total	718.2	48.8

\* Movement not allocated to the lane

## DOWNSTREAM LANE FLOW RATES FOR EXIT ROADS

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	180.8	16.5
Lane: 2	197.3	10.4
Total	378.1	26.9
Exit: SOUTHEAST		
Lane: 1	192.6	15.1
Lane: 2	126.6	6.7
Total	319.2	21.8
Exit: NORTHEAST		
Lane: 1	182.0	30.0
Total	182.0	30.0
Exit: NORTH		
Lane: 1	163.1	8.6
Lane: 2	253.0	14.3
Total	416.1	22.9
Exit: NORTHWEST		
Lane: 1	502.6	26.5
Lane: 2	215.6	22.3
Total	718.2	48.8
* Movement not allocated to the lane		

Unit Time for Volumes = 60 minutes

Peak Flow Period = 30 minutes

Flow Rates include effects of Flow Scale and Peak Flow Factor

[Go to Table Links \(Top\)](#)**Other**

## Model Settings Summary

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

Intersection ID: 1  
Roundabout

\* Basic Parameters:  
 Intersection Type: Roundabout  
 Driving on the left-hand side of the road  
 Input data specified in Metric units  
 Model Defaults: New South Wales  
 Peak Flow Period (for performance): 30 minutes  
 Unit time (for volumes): 60 minutes.  
 SIDRA Standard Delay model used  
 SIDRA Standard Queue model used  
 Level of Service based on: Delay (RTA NSW)  
 Queue percentile: 95%

[Go to Table Links \(Top\)](#)

## Diagnostics

Site:WD PM (NDR/Leeds) - proposed +bunnings + service centre

[Go to Table Links \(Top\)](#)Processed: 2 February 2016 1:43:51 PM  
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8000782, GEOLYSE PTY LTD, PLUS / 1PC**SIDRA  
INTERSECTION 6**

**DETAILED OUTPUT****Site: WD PM (NDR/Leeds) - existing**New Site  
Roundabout**OUTPUT TABLE LINKS****Roundabouts**

- Roundabout Basic Parameters
- Roundabout Circulating / Exiting Stream Parameters
- Roundabout Gap Acceptance Parameters
- Roundabout Flow Rates

**Movements**

- Intersection Negotiation Data
- Movement Capacity and Performance Parameters
- Fuel Consumption, Emissions and Cost

**Lanes**

- Lane Performance and Capacity Information
- Lane Delays
- Lane Queues
- Lane Queue Percentiles
- Lane Stops

**Flow Rates**

- Origin-Destination Flow Rates (Total)
- Origin-Destination Flow Rates by Movement Class
- Lane Flow Rates

**Other**

- Model Settings Summary
- Diagnostics

**Roundabouts**Roundabout Basic Parameters  
Site:WD PM (NDR/Leeds) - existingIntersection ID: 1  
Roundabout

Central Island Diam m	Circ Width m	Insc Diam. m	Entry Radius m	Entry Angle deg	Circ Lanes	Entry Lanes	Av.Entry Lane Width m
South: Leeds Parade south							
36.0	10.0	56.0	20.0	30.0	2	2	3.70
SouthEast: Northern Distributor east							
36.0	10.0	56.0	20.0	30.0	2	3	3.70
NorthEast: Hanrahan Pl							
36.0	10.0	56.0	20.0	30.0	2	1	3.70
North: Leeds Parade north							
36.0	10.0	56.0	20.0	30.0	2	2	3.70
NorthWest: Northern Distributor west							
36.0	10.0	56.0	20.0	30.0	2	2	3.70
Roundabout Capacity Model: SIDRA Standard							

[Go to Table Links \(Top\)](#)Roundabout Circulating / Exiting Stream Parameters  
Site:WD PM (NDR/Leeds) - existingIntersection ID: 1  
Roundabout

Dest	Turn	Lane No.	Lane Type	Opng Flow veh/h	HVE pcu/ veh	Adj. Flow pcu/h	%Near Lane Only	%Exit Flow Incl.	Cap. Const. Effect	O-D Factor	Aver Speed km/h	In-Bunch Headway sec	Prop. Bunched
South: Leeds Parade south													
NW	L1	1	Dominant	313	1.04	327	0.0	0.0	N	0.979	39.7	1.04	0.187

N	T1	2	Subdominant	313	1.04	327	0.0	0.0	N	0.979	39.7	1.04	0.187
NE	R1	2	Subdominant	313	1.04	327	0.0	0.0	N	0.979	39.7	1.04	0.187
SE	R3	2	Subdominant	313	1.04	327	0.0	0.0	N	0.979	39.7	1.04	0.187
-----													
SouthEast: Northern Distributor east													
S	L3	1	Excl. Slip	190	1.04	198	0.0	0.0	N	0.993	30.8	1.40	0.155
NW	T1	2	Dominant	233	1.06	248	0.0	0.0	N	0.993	30.2	1.53	0.205
NW	T1	3	Subdominant	233	1.06	248	0.0	0.0	N	0.993	30.2	1.53	0.205
N	R1	3	Subdominant	233	1.06	248	0.0	0.0	N	0.993	30.2	1.53	0.205
NE	R2	3	Subdominant	233	1.06	248	0.0	0.0	N	0.993	30.2	1.53	0.205
-----													
NorthEast: Hanrahan Pl													
SE	L2	1	Dominant	450	1.03	461	0.0	0.0	N	0.985	35.2	1.06	0.257
S	L1	1	Dominant	450	1.03	461	0.0	0.0	N	0.985	35.2	1.06	0.257
NW	R2	1	Dominant	450	1.03	461	0.0	0.0	N	0.985	35.2	1.06	0.257
N	R3	1	Dominant	450	1.03	461	0.0	0.0	N	0.985	35.2	1.06	0.257
-----													
North: Leeds Parade north													
NE	L3	1	Dominant	388	1.05	407	0.0	0.0	N	0.985	34.8	1.09	0.237
SE	L1	1	Dominant	388	1.05	407	0.0	0.0	N	0.985	34.8	1.09	0.237
S	T1	1	Dominant	388	1.05	407	0.0	0.0	N	0.985	34.8	1.09	0.237
S	T1	2	Subdominant	388	1.05	407	0.0	0.0	N	0.985	34.8	1.09	0.237
NW	R3	2	Subdominant	388	1.05	407	0.0	0.0	N	0.985	34.8	1.09	0.237
-----													
NorthWest: Northern Distributor west													
N	L3	1	Dominant	55	1.11	61	0.0	0.0	N	0.992	30.7	2.00	0.072
NE	L2	1	Dominant	55	1.11	61	0.0	0.0	N	0.992	30.7	2.00	0.072
SE	T1	1	Dominant	55	1.11	61	0.0	0.0	N	0.992	30.7	2.00	0.072
SE	T1	2	Subdominant	55	1.11	61	0.0	0.0	N	0.992	30.7	2.00	0.072
S	R1	2	Subdominant	55	1.11	61	0.0	0.0	N	0.992	30.7	2.00	0.072
-----													
Roundabout Capacity Model: SIDRA Standard													

[Go to Table Links \(Top\)](#)

### Roundabout Gap Acceptance Parameters Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

Dest	Turn	Lane No.	Lane Type	In-Bunch Headway sec	Priority Sharing	Critical Gap		Follow-up Headway sec
						Headway sec	Dist m	
-----								
South: Leeds Parade south								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
NW	L1	1	Dominant	1.04	N	3.73	41.1	2.24
N	T1	2	Subdominant	1.04	N	5.36	59.1	3.21
NE	R1	2	Subdominant	1.04	N	7.85	86.5	4.70
SE	R3	2	Subdominant	1.04	N	5.36	59.1	3.21
-----								
SouthEast: Northern Distributor east								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
S	L3	1	Excl. Slip	1.40	Y	3.26	27.9	1.91
NW	T1	2	Dominant	1.53	Y	3.20	26.8	1.89
NW	T1	3	Subdominant	1.53	N	3.89	32.6	2.30
N	R1	3	Subdominant	1.53	N	3.89	32.6	2.30
NE	R2	3	Subdominant	1.53	N	5.69	47.7	3.36
-----								
NorthEast: Hanrahan Pl								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
SE	L2	1	Dominant	1.06	N	6.11	59.7	3.75
S	L1	1	Dominant	1.06	N	6.11	59.7	3.75
NW	R2	1	Dominant	1.06	N	6.11	59.7	3.75
N	R3	1	Dominant	1.06	N	6.11	59.7	3.75
-----								
North: Leeds Parade north								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
NE	L3	1	Dominant	1.09	N	5.30	51.3	3.23
SE	L1	1	Dominant	1.09	N	3.62	35.0	2.20
S	T1	1	Dominant	1.09	N	3.62	35.0	2.20
S	T1	2	Subdominant	1.09	N	4.07	39.3	2.47
NW	R3	2	Subdominant	1.09	N	4.07	39.3	2.47
-----								
NorthWest: Northern Distributor west								
Environment Factor: 1.00								
Entry/Circ. Flow Adjustment: Medium								
N	L3	1	Dominant	2.00	Y	3.97	33.8	2.25
NE	L2	1	Dominant	2.00	N	5.80	49.5	3.30
SE	T1	1	Dominant	2.00	Y	3.97	33.8	2.25
SE	T1	2	Subdominant	2.00	Y	4.38	37.3	2.49
S	R1	2	Subdominant	2.00	Y	4.38	37.3	2.49
-----								

Roundabout Capacity Model: SIDRA Standard  
 Priority sharing means Follow-up Headway plus Intra-bunch Headway  
 is larger than the Critical Gap.

Dist (Distance): Spacing, i.e. distance between the front ends of two  
 successive vehicles across all lanes in the circulating  
 or exiting stream

[Go to Table Links \(Top\)](#)

## Roundabout Flow Rates

Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
 Roundabout

### CIRCULATING LANE FLOW RATES

Lane No.	Circulating Flow Rates		
	veh/h	pcu/h	Percent
South: Leeds Parade south			
1	151	154	47.2%
2	162	173	52.8%
Total	313	327	
SouthEast: Northern Distributor east			
1	32	33	13.3%
2	201	215	86.7%
Total	233	248	
NorthEast: Hanrahan Pl			
1	207	212	46.0%
2	243	249	54.0%
Total	450	461	
North: Leeds Parade north			
1	214	229	56.2%
2	174	179	43.8%
Total	388	408	
NorthWest: Northern Distributor west			
1	0	0	0.0%
2	55	61	100.0%
Total	55	61	

### APPROACH LANE FLOW RATES

Lane No.	Approach Flows (veh/h)		
	Out	To Downst	Total
South: Leeds Parade south			
1	271	0	271
2	0	51	51
Total	271	51	322
SouthEast: Northern Distributor east			
1	52	0	52
2	0	151	151
3	0	119	119
Total	52	270	322
NorthEast: Hanrahan Pl			
1	5	19	24
Total	5	19	24
North: Leeds Parade north			
1	1	43	44
2	0	39	39
Total	1	82	83
NorthWest: Northern Distributor west			
1	13	175	188
2	0	174	174
Total	13	349	362

### EXITING LANE FLOW RATES

Lane No.	Exiting Flow Rates		
	veh/h	pcu/h	Percent
SouthEast: Northern Distributor east			



1	39	44	22.0%
2	151	155	78.0%
Total	190	199	

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## Movements

### Intersection Negotiation Data

Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

#### INTERSECTION NEGOTIATION DATA

From Approach	To Exit	Turn	Negn Radius m	Negn Speed km/h	Negn Dist. m	Appr. Dist. m	Downstream m	Distance User Spec?
South: Leeds Parade south								
	NorthWest	L1	68.6	41.9	39.9	500	286	No
	North	T1	68.6	41.9	53.2	500	479	No
	NorthEast	R1	22.0	27.2	69.1	500	524	No
	SouthEast	R3	22.0	27.2	103.7	500	516	No
SouthEast: Northern Distributor east								
	South	L3	33.0	31.7	11.0	500	445	No
	NorthWest	T1	68.6	41.9	53.2	500	289	No
	North	R1	22.0	27.2	69.1	500	446	No
	NorthEast	R2	22.0	27.2	86.4	500	331	No
NorthEast: Hanrahan Pl								
	SouthEast	L2	46.6	36.2	21.6	500	2919	No
	South	L1	68.6	41.9	39.9	500	3040	No
	NorthWest	R2	22.0	27.2	86.4	500	1711	No
	North	R3	22.0	27.2	103.7	500	2820	No
North: Leeds Parade north								
	NorthEast	L3	30.6	30.8	11.0	500	363	No
	SouthEast	L1	68.6	41.9	39.9	500	441	No
	South	T1	68.6	41.9	53.2	500	481	No
	NorthWest	R3	22.0	27.2	103.7	500	364	No
NorthWest: Northern Distributor west								
	North	L3	30.6	30.8	11.0	500	412	No
	NorthEast	L2	42.9	35.0	21.7	500	446	No
	SouthEast	T1	68.6	41.9	53.2	500	482	No
	South	R1	22.0	27.2	69.1	500	485	No

Maximum Negotiation (Design) Speed = 50.0 km/h

Downstream distance is distance travelled from the stopline until exit cruise speed is reached (includes negotiation distance). Acceleration distance is weighted for light and heavy vehicles. The same distance applies for both stopped and unstopped vehicles.

#### MOVEMENT SPEEDS AND GEOMETRIC DELAY

						Queue Move-up				
Mov	Turn	App. Speeds		Exit Speeds				Av. Section Spd		Geom
ID		Cruise	Negn	Negn	Cruise	1st Grn	2nd Grn	Running	Overall	Delay sec
South: Leeds Parade south										
1a	L1	80.0	41.9	41.9	70.0	25.6		58.8	58.8	8.8
2	T1	80.0	41.9	41.9	80.0	22.5		51.5	51.2	11.6
3a	R1	80.0	27.2	27.2	50.0	22.5		51.5	51.2	95.5
3b	R3	80.0	27.2	27.2	80.0	22.5		51.5	51.2	22.9
SouthEast: Northern Distributor east										
21b	L3	80.0	31.7	31.7	80.0	31.7		58.6	58.6	14.7
22	T1	80.0	41.9	41.9	70.0	30.5		59.5	59.5	8.9
23a	R1	80.0	27.2	27.2	80.0	29.0		59.2	59.2	19.9
23	R2	80.0	27.2	27.2	50.0	29.0		59.2	59.2	23.8
NorthEast: Hanrahan Pl										
24	L2	50.0	36.2	36.2	80.0	23.9		49.2	49.2	34.5
24a	L1	50.0	41.9	41.9	80.0	23.9		49.2	49.2	26.5
26	R2	50.0	27.2	27.2	70.0	23.9		49.2	49.2	38.1
26b	R3	50.0	27.2	27.2	80.0	23.9		49.2	49.2	47.4

North: Leeds Parade north									
7b	L3	80.0	30.8	30.8	50.0	23.8	59.7	59.7	117.7
7a	L1	80.0	41.9	41.9	80.0	23.8	59.7	59.7	11.5
8	T1	80.0	41.9	41.9	80.0	23.6	58.1	58.1	11.6
9b	R3	80.0	27.2	27.2	70.0	22.5	52.2	52.2	19.3

NorthWest: Northern Distributor west									
27b	L3	70.0	30.8	30.8	80.0	40.7	57.6	57.6	13.7
27	L2	70.0	35.0	35.0	50.0	40.7	57.6	57.6	131.9
28	T1	70.0	41.9	41.9	80.0	39.0	56.8	56.8	10.3
29a	R1	70.0	27.2	27.2	80.0	29.8	53.4	53.4	18.8

"Running Speed" is the average speed excluding stopped periods.

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## Movement Capacity and Performance Parameters

Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

### MOVEMENT CAPACITY PARAMETERS

Mov ID	Turn	Mov Cl.	Arv Flow veh/h	Opng Flow veh/h	Movement Adjust. Flow pcu/h	Total Cap. veh/h	Prac. Deg. Satn xp	Prac. Spare Cap. %	Deg. Satn x		
South: Leeds Parade south											
1a	L1	#	271	313	327	1251	0.85	292	0.217*		
2	T1	#	13	313	327	181	0.85	1086	0.072		
3a	R1	LV	0	0	0	0.000	0.000	0	0.85	****	0.000
3a	R1	HV	8	37	0	0.000	0.000	112	0.85	1086	0.072
3b	R3	#	30	313	327	419	0.85	1086	0.072		
SouthEast: Northern Distributor east											
21b	L3	#	52	190	198	1652	0.85	2600	0.031		
22	T1	#	267	466	496	2871	0.85	814	0.093		
23a	R1	#	2	233	248	22	0.85	814	0.093		
23	R2	LV	0	0	0	0.000	0.000	0	0.85	****	0.000
23	R2	HV	1	11	0	0.000	0.000	11	0.85	814	0.093
NorthEast: Hanrahan Pl											
24	L2	LV	0	0	0	0.000	0.000	0	0.85	****	0.000
24	L2	HV	5	118	0	0.000	0.000	118	0.85	1903	0.042
24a	L1	LV	0	0	0	0.000	0.000	0	0.85	****	0.000
24a	L1	HV	7	165	0	0.000	0.000	165	0.85	1903	0.042
26	R2	LV	0	0	0	0.000	0.000	0	0.85	****	0.000
26	R2	HV	11	259	0	0.000	0.000	259	0.85	1903	0.042
26b	R3	LV	0	0	0	0.000	0.000	0	0.85	****	0.000
26b	R3	HV	1	24	0	0.000	0.000	24	0.85	1903	0.042
North: Leeds Parade north											
7b	L3	LV	0	0	0	0.000	0.000	0	0.85	****	0.000
7b	L3	HV	1	27	0	0.000	0.000	27	0.85	2205	0.037
7a	L1	#	11	388	407	298	0.85	2205	0.037		
8	T1	#	40	776	814	1085	0.85	2205	0.037		
9b	R3	#	31	388	407	841	0.85	2205	0.037		
NorthWest: Northern Distributor west											
27b	L3	#	13	55	61	101	0.85	564	0.128		
27	L2	LV	0	0	0	0.000	0.000	0	0.85	****	0.000
27	L2	HV	11	86	0	0.000	0.000	86	0.85	564	0.128
28	T1	#	195	110	122	1522	0.85	564	0.128		
29a	R1	#	143	55	61	1116	0.85	564	0.128		

\* Maximum degree of saturation

# Combined Movement Capacity parameters are shown for all Movement Classes.

### MOVEMENT PERFORMANCE

Mov ID	Turn	Total Delay (veh-h/h)	Total Delay (pers-h/h)	Aver. Delay (sec)	Eff. Stop Rate	Total Stops	Perf. Index	Tot.Trav. Distance (veh-km/h)	Tot.Trav. Time (veh-h/h)	Aver. Speed (km/h)
South: Leeds Parade south										
1a	L1	0.04	0.04	9.8	1.19	321.6	9.69	383.6	6.5	58.8
2	T1	0.00	0.00	22.3	1.50	19.5	0.96	23.5	0.5	51.2
3a	R1	0.22	0.26	22.3	1.50	12.0	0.91	14.5	0.3	51.2
3b	R3	0.01	0.01	22.3	1.50	45.1	1.80	54.3	1.1	51.2
SouthEast: Northern Distributor east										
21b	L3	0.01	0.01	15.1	1.28	66.4	2.08	84.9	1.4	58.6
22	T1	0.04	0.04	9.7	1.10	292.4	8.45	380.3	6.4	59.5
23a	R1	0.00	0.00	10.0	1.12	2.2	0.72	2.9	0.0	59.2

23	R2	0.01	0.01	10.0	1.12	1.1	0.36	1.4	0.0	59.2
-----										
NorthEast: Hanrahan Pl										
24	L2	0.05	0.06	37.8	1.48	7.4	0.85	28.8	0.6	49.2
24a	L1	0.06	0.07	37.8	1.48	10.4	1.11	40.3	0.8	49.2
26	R2	0.13	0.15	37.8	1.48	16.3	1.74	63.3	1.3	49.2
26b	R3	0.01	0.02	37.8	1.48	1.5	0.28	5.8	0.1	49.2
-----										
North: Leeds Parade north										
7b	L3	0.03	0.04	12.8	1.23	1.2	0.21	1.6	0.0	59.7
7a	L1	0.00	0.00	12.8	1.23	13.5	0.61	18.0	0.3	59.7
8	T1	0.01	0.01	14.0	1.27	50.9	1.61	65.2	1.1	58.1
9b	R3	0.01	0.01	19.0	1.46	45.3	1.46	49.5	0.9	52.2
-----										
NorthWest: Northern Distributor west										
27b	L3	0.00	0.00	11.0	1.07	13.9	1.53	21.5	0.4	57.6
27	L2	0.40	0.48	11.0	1.07	11.8	1.65	18.2	0.3	57.6
28	T1	0.03	0.03	12.1	1.12	217.7	8.03	324.8	5.7	56.8
29a	R1	0.04	0.05	17.5	1.37	195.9	7.12	244.4	4.6	53.4
-----										

[Go to Table Links \(Top\)](#)

### Fuel Consumption, Emissions and Cost Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

#### FUEL CONSUMPTION, EMISSIONS AND COST (TOTAL)

Mov ID	Turn	Cost Total \$/h	Fuel Total L/h	CO2 Total kg/h	CO Total kg/h	HC Total kg/h	NOX Total kg/h
-----							
South: Leeds Parade south							
1a	L1	133.37	14.2	33.5	0.09	0.009	0.114
2	T1	10.06	1.4	3.3	0.01	0.001	0.015
3a	R1	6.19	0.8	2.0	0.00	0.000	0.009
3b	R3	23.22	3.1	7.5	0.02	0.002	0.035
		172.84	19.5	46.3	0.11	0.012	0.174
-----							
SouthEast: Northern Distributor east							
21b	L3	31.39	3.6	8.5	0.02	0.002	0.032
22	T1	131.13	14.2	33.5	0.09	0.009	0.116
23a	R1	1.00	0.1	0.3	0.00	0.000	0.001
23	R2	0.50	0.1	0.1	0.00	0.000	0.000
		164.02	17.9	42.4	0.11	0.011	0.150
-----							
NorthEast: Hanrahan Pl							
24	L2	15.85	3.6	9.4	0.01	0.001	0.053
24a	L1	22.20	5.0	13.1	0.02	0.001	0.075
26	R2	34.88	7.8	20.6	0.03	0.002	0.118
26b	R3	3.17	0.7	1.9	0.00	0.000	0.011
		76.10	17.0	44.9	0.06	0.004	0.257
-----							
North: Leeds Parade north							
7b	L3	0.60	0.1	0.2	0.00	0.000	0.001
7a	L1	6.64	0.8	1.9	0.00	0.000	0.008
8	T1	24.16	2.8	6.7	0.02	0.002	0.026
9b	R3	18.76	1.9	4.4	0.01	0.001	0.015
		50.16	5.6	13.2	0.03	0.003	0.049
-----							
NorthWest: Northern Distributor west							
27b	L3	8.24	1.0	2.5	0.01	0.001	0.011
27	L2	6.97	0.9	2.1	0.00	0.000	0.009
28	T1	124.61	15.4	36.7	0.08	0.008	0.153
29a	R1	95.37	10.4	24.5	0.06	0.007	0.089
		235.19	27.7	65.8	0.15	0.016	0.261
-----							
INTERSECTION:		691.40	87.7	212.4	0.47	0.047	0.890
-----							

#### FUEL CONSUMPTION, EMISSIONS AND COST (RATE)

Mov ID	Turn	Cost Rate \$/km	Fuel Rate L/100km	CO2 Rate g/km	CO Rate g/km	HC Rate g/km	NOX Rate g/km
-----							
South: Leeds Parade south							
1a	L1	0.35	3.7	87.3	0.23	0.024	0.298

2	T1	0.43	5.8	138.9	0.29	0.029	0.650
3a	R1	0.43	5.8	138.9	0.29	0.029	0.650
3b	R3	0.43	5.8	138.9	0.29	0.029	0.650
		0.36	4.1	97.3	0.24	0.025	0.366
-----							
SouthEast: Northern Distributor east							
21b	L3	0.37	4.2	100.4	0.25	0.025	0.379
22	T1	0.34	3.7	88.1	0.23	0.024	0.306
23a	R1	0.35	3.8	89.8	0.23	0.024	0.318
23	R2	0.35	3.8	89.8	0.23	0.024	0.318
		0.35	3.8	90.3	0.23	0.024	0.319
-----							
NorthEast: Hanrahan Pl							
24	L2	0.55	12.3	324.8	0.41	0.031	1.856
24a	L1	0.55	12.3	324.8	0.41	0.031	1.856
26	R2	0.55	12.3	324.8	0.41	0.031	1.856
26b	R3	0.55	12.3	324.8	0.41	0.031	1.856
		0.55	12.3	324.8	0.41	0.031	1.856
-----							
North: Leeds Parade north							
7b	L3	0.37	4.5	105.7	0.25	0.025	0.423
7a	L1	0.37	4.5	105.7	0.25	0.025	0.423
8	T1	0.37	4.3	102.7	0.25	0.025	0.399
9b	R3	0.38	3.8	89.9	0.24	0.027	0.296
		0.37	4.2	98.4	0.24	0.026	0.365
-----							
NorthWest: Northern Distributor west							
27b	L3	0.38	4.8	115.4	0.26	0.026	0.490
27	L2	0.38	4.8	115.4	0.26	0.026	0.490
28	T1	0.38	4.7	112.9	0.26	0.026	0.470
29a	R1	0.39	4.2	100.4	0.25	0.027	0.366
		0.39	4.6	108.1	0.25	0.026	0.429
-----							
INTERSECTION:		0.63	8.0	193.8	0.43	0.043	0.812
-----							

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## Lanes

### Lane Performance and Capacity Information Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

#### LANE PERFORMANCE

	Flow	Cap	Deg.	Aver.	Eff.	Q u e u e		
Lane			Satn	Delay	Stop	95% Back		Lane
No.	veh/h	veh/h	x	sec	Rate	veh	m	Length
								m
-----								
South: Leeds Parade south								
1	271	1251	0.217	9.8	0.59	1.0	7.4	60.0
2	51	712	0.072	22.3	0.75	0.3	2.3	500.0
-----								
SouthEast: Northern Distributor east								
1	52	1652	0.031	15.1	0.64	0.1	1.0	20.0
2	151	1619	0.093	9.4	0.54	0.4	3.0	120.0
3	119	1284	0.093	10.0	0.56	0.4	2.9	500.0
-----								
NorthEast: Hanrahan Pl								
1	24	566	0.042	37.8	0.74	0.2	2.0	190.0
-----								
North: Leeds Parade north								
1	44	1199	0.037	12.8	0.61	0.1	1.1	100.0
2	39	1052	0.037	19.0	0.73	0.1	1.1	500.0
-----								
NorthWest: Northern Distributor west								
1	188	1466	0.128	11.0	0.53	0.6	4.9	60.0
2	174	1360	0.128	17.5	0.69	0.6	4.7	500.0
-----								

#### LANE FLOW AND CAPACITY INFORMATION

Lane No.	Total Arv Flow (veh/h)	Min Cap veh/h	Tot Cap veh/h	Deg. Satn x	Lane Util %
----------	------------------------	---------------	---------------	-------------	-------------

-----					
South: Leeds Parade south					
1	271	150	1251	0.217	100
2	51	51	712	0.072	33P
-----					
SouthEast: Northern Distributor east					
1	52	52	1652	0.031	100
2	151	150	1619	0.093	100
3	119	119	1284	0.093	100
-----					
NorthEast: Hanrahan Pl					
1	24	24	566	0.042	100
-----					
North: Leeds Parade north					
1	44	44	1199	0.037	100
2	39	39	1052	0.037	100
-----					
NorthWest: Northern Distributor west					
1	188	150	1466	0.128	100
2	174	150	1360	0.128	100
-----					
P Lane under-utilisation found by the "Program". This includes cases where the value of lane under-utilisation due to downstream effects has been modified by the program during lane flow calculations (e.g. a de facto exclusive lane has been found).					
The capacity value for priority and continuous movements is obtained by adjusting the basic saturation flow for heavy vehicle and turning vehicle effects. Saturation flow scale applies if specified.					

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## Lane Delays

Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

### LANE DELAYS

-----											
Lane No.	Deg. Satn	Prog. Factor	Stop-line Delay			Delay (seconds/veh)					
	x		1st d1	2nd d2	Total dSL	Acc. Dec. dn	Queuing Total dq	MvUp dqm	Stopd (Idle) di	Geom dig	Control dic
-----											
South: Leeds Parade south											
1	0.217	1.000	1.0	0.0	1.0	2.9	0.0	0.0	0.0	8.8	9.8
2	0.072	1.000	2.2	0.0	2.2	2.8	0.3	0.0	0.3	20.1	22.3
-----											
SouthEast: Northern Distributor east											
1	0.031	1.000	0.4	0.0	0.4	1.5	0.0	0.0	0.0	14.7	15.1
2	0.093	1.000	0.5	0.0	0.5	2.1	0.0	0.0	0.0	8.9	9.4
3	0.093	1.000	0.8	0.0	0.8	2.3	0.0	0.0	0.0	9.2	10.0
-----											
NorthEast: Hanrahan Pl											
1	0.042	1.000	3.4	0.0	3.4	9.9	0.0	0.0	0.0	34.4	37.8
-----											
North: Leeds Parade north											
1	0.037	1.000	1.0	0.0	1.0	2.8	0.0	0.0	0.0	11.8	12.8
2	0.037	1.000	1.3	0.0	1.3	1.9	0.0	0.0	0.0	17.7	19.0
-----											
NorthWest: Northern Distributor west											
1	0.128	1.000	0.2	0.0	0.2	1.4	0.0	0.0	0.0	10.8	11.0
2	0.128	1.000	0.2	0.0	0.2	0.9	0.0	0.0	0.0	17.3	17.5
-----											
SIDRA Standard Delay Model is used. Control Delay is the sum of Stop-line Delay and Geometric Delay.											
dSL: Stop-line delay (=d1+d2)											
dn: Average stop-start delay for all vehicles queued and unqueued											
dq: Queuing delay (the part of the stop-line delay that includes stopped delay and queue move-up delay)											
dqm: Queue move-up delay											
di: Stopped delay (stopped (idling) time at near-zero speed)											
dig: Geometric delay											
dic: Control delay											

### LANE DELAY PERCENTILES

Lane No.	Deg. Satn	Percentile Delay						
	x	50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade south								
1	0.217	9.8	10.0	10.3	10.5	10.7	10.8	10.9
2	0.072	22.3	22.7	23.3	23.6	24.0	24.3	24.5

SouthEast: Northern Distributor east								
1	0.031	15.1	15.1	15.2	15.3	15.4	15.4	15.5
2	0.093	9.4	9.5	9.7	9.8	9.9	9.9	10.0
3	0.093	10.0	10.1	10.3	10.5	10.6	10.7	10.8
-----								
NorthEast: Hanrahan Pl								
1	0.042	37.8	38.4	39.4	39.9	40.5	41.0	41.3
-----								
North: Leeds Parade north								
1	0.037	12.8	13.0	13.3	13.4	13.6	13.8	13.9
2	0.037	19.0	19.2	19.6	19.8	20.0	20.2	20.3
-----								
NorthWest: Northern Distributor west								
1	0.128	11.0	11.1	11.1	11.2	11.2	11.2	11.3
2	0.128	17.5	17.5	17.6	17.6	17.7	17.7	17.7
-----								

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## Lane Queues

Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

### LANE QUEUES (VEHICLES)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (veh)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Nc	Queue 95%
	x		No	Nb1	Nb2	Nb	95%					95%
South: Leeds Parade south												
1	0.217	1.000	0.0	0.4	0.0	0.4	1.0	0.05	0.0	100.0	0.1	0.1
2	0.072	1.000	0.0	0.1	0.0	0.1	0.3	0.00	0.0	100.0	0.0	0.1
-----												
SouthEast: Northern Distributor east												
1	0.031	1.000	0.0	0.1	0.0	0.1	0.1	0.02	0.0	100.0	0.0	0.0
2	0.093	1.000	0.0	0.2	0.0	0.2	0.4	0.01	0.0	100.0	0.0	0.0
3	0.093	1.000	0.0	0.2	0.0	0.2	0.4	0.00	0.0	100.0	0.0	0.0
-----												
NorthEast: Hanrahan Pl												
1	0.042	1.000	0.0	0.1	0.0	0.1	0.2	0.00	0.0	100.0	0.0	0.0
-----												
North: Leeds Parade north												
1	0.037	1.000	0.0	0.1	0.0	0.1	0.1	0.00	0.0	100.0	0.0	0.0
2	0.037	1.000	0.0	0.1	0.0	0.1	0.1	0.00	0.0	100.0	0.0	0.0
-----												
NorthWest: Northern Distributor west												
1	0.128	1.000	0.0	0.3	0.0	0.3	0.6	0.03	0.0	100.0	0.0	0.0
2	0.128	1.000	0.0	0.3	0.0	0.3	0.6	0.00	0.0	100.0	0.0	0.0
-----												

### LANE QUEUES (DISTANCE)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (m)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Queue	
	x		No	Nb1	Nb2	Nb	95%		%	%	Nc	95%
South: Leeds Parade south												
1	0.217	1.000	0.0	3.0	0.0	3.0	7.4	0.05	0.0	100.0	0.6	1.0
2	0.072	1.000	0.0	0.9	0.0	0.9	2.3	0.00	0.0	100.0	0.3	0.5
SouthEast: Northern Distributor east												
1	0.031	1.000	0.0	0.4	0.0	0.4	1.0	0.02	0.0	100.0	0.0	0.1
2	0.093	1.000	0.0	1.2	0.0	1.2	3.0	0.01	0.0	100.0	0.2	0.3
3	0.093	1.000	0.0	1.2	0.0	1.2	2.9	0.00	0.0	100.0	0.2	0.3
NorthEast: Hanrahan Pl												
1	0.042	1.000	0.0	0.8	0.0	0.8	2.0	0.00	0.0	100.0	0.3	0.5
North: Leeds Parade north												
1	0.037	1.000	0.0	0.4	0.0	0.4	1.1	0.00	0.0	100.0	0.1	0.2
2	0.037	1.000	0.0	0.4	0.0	0.4	1.1	0.00	0.0	100.0	0.1	0.2
NorthWest: Northern Distributor west												
1	0.128	1.000	0.0	2.0	0.0	2.0	4.9	0.03	0.0	100.0	0.1	0.1
2	0.128	1.000	0.0	1.9	0.0	1.9	4.7	0.00	0.0	100.0	0.1	0.1

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## Lane Queue Percentiles

Site:WD PM (NDR/Leeds) - existing



Intersection ID: 1  
Roundabout

## LANE QUEUE PERCENTILES (VEHICLES)

Lane No.	Deg. Satn x	Percentile Back of Queue (veh)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade south								
1	0.217	0.4	0.5	0.7	0.9	1.0	1.1	1.2
2	0.072	0.1	0.1	0.2	0.2	0.3	0.3	0.3
SouthEast: Northern Distributor east								
1	0.031	0.1	0.1	0.1	0.1	0.1	0.1	0.2
2	0.093	0.2	0.2	0.3	0.4	0.4	0.5	0.5
3	0.093	0.2	0.2	0.3	0.3	0.4	0.4	0.5
NorthEast: Hanrahan Pl								
1	0.042	0.1	0.1	0.1	0.1	0.2	0.2	0.2
North: Leeds Parade north								
1	0.037	0.1	0.1	0.1	0.1	0.1	0.2	0.2
2	0.037	0.1	0.1	0.1	0.1	0.1	0.2	0.2
NorthWest: Northern Distributor west								
1	0.128	0.3	0.3	0.5	0.6	0.6	0.7	0.8
2	0.128	0.3	0.3	0.5	0.5	0.6	0.7	0.8

## LANE QUEUE PERCENTILES (DISTANCE)

Lane No.	Deg. Satn x	Percentile Back of Queue (metres)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade south								
1	0.217	3.0	3.8	5.4	6.3	7.4	8.2	8.8
2	0.072	0.9	1.2	1.7	2.0	2.3	2.6	2.8
SouthEast: Northern Distributor east								
1	0.031	0.4	0.5	0.7	0.8	1.0	1.1	1.1
2	0.093	1.2	1.6	2.2	2.6	3.0	3.3	3.6
3	0.093	1.2	1.5	2.2	2.5	2.9	3.3	3.5
NorthEast: Hanrahan Pl								
1	0.042	0.8	1.1	1.5	1.7	2.0	2.2	2.4
North: Leeds Parade north								
1	0.037	0.4	0.6	0.8	0.9	1.1	1.2	1.3
2	0.037	0.4	0.5	0.8	0.9	1.1	1.2	1.3
NorthWest: Northern Distributor west								
1	0.128	2.0	2.6	3.6	4.2	4.9	5.5	5.9
2	0.128	1.9	2.4	3.4	4.0	4.7	5.2	5.6

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## Lane Stops

Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

Lane No.	Deg. Satn x	Prog. Factor	Effective Stop Rate				Queue Total Stops H	Total Move-up Rate hqm	Queue Move-ups Hqm	Prop. Queued pq
			hel	he2	hig	h				
South: Leeds Parade south										
1	0.217	1.000	0.29	0.00	0.31	0.59	160.8	0.00	0.0	0.41
2	0.072	1.000	0.33	0.00	0.42	0.75	38.3	0.00	0.0	0.45
SouthEast: Northern Distributor east										
1	0.031	1.000	0.12	0.00	0.52	0.64	33.2	0.00	0.0	0.26
2	0.093	1.000	0.17	0.00	0.36	0.54	80.7	0.00	0.0	0.31
3	0.093	1.000	0.20	0.00	0.36	0.56	67.2	0.00	0.0	0.33
NorthEast: Hanrahan Pl										
1	0.042	1.000	0.39	0.00	0.35	0.74	17.8	0.00	0.0	0.50

North: Leeds Parade north										
1	0.037	1.000	0.25	0.00	0.37	0.61	27.1	0.00	0.0	0.39
2	0.037	1.000	0.27	0.00	0.46	0.73	28.3	0.00	0.0	0.41
-----										
NorthWest: Northern Distributor west										
1	0.128	1.000	0.07	0.00	0.47	0.53	100.3	0.00	0.0	0.18
2	0.128	1.000	0.07	0.00	0.61	0.69	119.3	0.00	0.0	0.19
-----										
hig is the average value for all movements in a shared lane										
hqm is average queue move-up rate for all vehicles queued and unqueued										

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## Flow Rates

### Origin-Destination Flow Rates (Total) Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

#### TOTAL FLOW RATES (ALL MOVEMENT CLASSES)

-----					
From SOUTH To:	NW	N	NE	SE	
Turn:	L1	T1	R1	R3	TOT
Flow Rate	271.0	13.0	8.0	30.0	322.0
%HV (all designations)	5.0	5.0	100.0	5.0	7.4
-----					
From SOUTHEAST To:	S	NW	N	NE	
Turn:	L3	T1	R1	R2	TOT
Flow Rate	52.0	267.0	2.0	1.0	322.0
%HV (all designations)	5.0	5.0	5.0	100.0	5.3
-----					
From NORTHEAST To:	SE	S	NW	N	
Turn:	L2	L1	R2	R3	TOT
Flow Rate	5.0	7.0	11.0	1.0	24.0
%HV (all designations)	100.0	100.0	100.0	100.0	100.0
-----					
From NORTH To:	NE	SE	S	NW	
Turn:	L3	L1	T1	R3	TOT
Flow Rate	1.0	11.0	40.0	31.0	83.0
%HV (all designations)	100.0	5.0	5.0	5.0	6.1
-----					
From NORTHWEST To:	N	NE	SE	S	
Turn:	L3	L2	T1	R1	TOT
Flow Rate	13.0	11.0	195.0	143.0	362.0
%HV (all designations)	5.0	100.0	5.0	5.0	7.9
-----					

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### Origin-Destination Flow Rates by Movement Class Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

#### FLOW RATES FOR Light Vehicles

-----					
From SOUTH To:	NW	N	NE	SE	
Turn:	L1	T1	R1	R3	TOT
Flow Rate - Veh	257.5	12.4	0.0	28.5	298.3
Mov Class %	95.0	95.0	0.0	95.0	92.6
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From SOUTHEAST To:	S	NW	N	NE	
Turn:	L3	T1	R1	R2	TOT
Flow Rate - Veh	49.4	253.6	1.9	0.0	304.9
Mov Class %	95.0	95.0	95.0	0.0	94.7
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTHEAST To:	SE	S	NW	N	
Turn:	L2	L1	R2	R3	TOT
Flow Rate - Veh	0.0	0.0	0.0	0.0	0.0
Mov Class %	0.0	0.0	0.0	0.0	0.0
Flow Scale - Fixed	1.00	1.00	1.00	1.00	

Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTH To:	NE	SE	S	NW	
Turn:	L3	L1	T1	R3	TOT
-----					
Flow Rate - Veh	0.0	10.4	38.0	29.5	77.9
Mov Class %	0.0	95.0	95.0	95.0	93.9
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTHWEST To:	N	NE	SE	S	
Turn:	L3	L2	T1	R1	TOT
-----					
Flow Rate - Veh	12.4	0.0	185.2	135.9	333.4
Mov Class %	95.0	0.0	95.0	95.0	92.1
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
FLOW RATES FOR Heavy Vehicles					
-----					
From SOUTH To:	NW	N	NE	SE	
Turn:	L1	T1	R1	R3	TOT
-----					
Flow Rate - Veh	13.6	0.6	8.0	1.5	23.7
Mov Class %	5.0	5.0	100.0	5.0	7.4
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From SOUTHEAST To:	S	NW	N	NE	
Turn:	L3	T1	R1	R2	TOT
-----					
Flow Rate - Veh	2.6	13.4	0.1	1.0	17.1
Mov Class %	5.0	5.0	5.0	100.0	5.3
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTHEAST To:	SE	S	NW	N	
Turn:	L2	L1	R2	R3	TOT
-----					
Flow Rate - Veh	5.0	7.0	11.0	1.0	24.0
Mov Class %	100.0	100.0	100.0	100.0	100.0
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTH To:	NE	SE	S	NW	
Turn:	L3	L1	T1	R3	TOT
-----					
Flow Rate - Veh	1.0	0.6	2.0	1.5	5.1
Mov Class %	100.0	5.0	5.0	5.0	6.1
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					
From NORTHWEST To:	N	NE	SE	S	
Turn:	L3	L2	T1	R1	TOT
-----					
Flow Rate - Veh	0.6	11.0	9.8	7.2	28.5
Mov Class %	5.0	100.0	5.0	5.0	7.9
Flow Scale - Fixed	1.00	1.00	1.00	1.00	
Flow Scale - Var	1.00	1.00	1.00	1.00	
Peak Flow Factor	1.00	1.00	1.00	1.00	
-----					

[Go to Table Links \(Top\)](#)

### Lane Flow Rates

Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

#### LANE FLOW RATES AT STOP LINE

From SOUTH To:	NW	N	NE	SE	
Turn:	L1	T1	R1	R3	TOT
-----					
Lane 1					
LV	257.5	*	*	*	257.5
HV	13.6	*	*	*	13.6
Total	271.0	*	*	*	271.0

Lane 2					
LV	*	12.4	*	28.5	40.8
HV	*	0.6	8.0	1.5	10.1
Total	*	13.0	8.0	30.0	51.0
Approach	271.0	13.0	8.0	30.0	322.0
From SOUTHEAST To:	S	NW	N	NE	
Turn:	L3	T1	R1	R2	TOT
Lane 1					
LV	49.4	*	*	*	49.4
HV	2.6	*	*	*	2.6
Total	52.0	*	*	*	52.0
Lane 2					
LV	*	143.0	*	*	143.0
HV	*	7.5	*	*	7.5
Total	*	150.6	*	*	150.6
Lane 3					
LV	*	110.6	1.9	*	112.5
HV	*	5.8	0.1	1.0	6.9
Total	*	116.4	2.0	1.0	119.4
Approach	52.0	267.0	2.0	1.0	322.0
From NORTHEAST To:	SE	S	NW	N	
Turn:	L2	L1	R2	R3	TOT
Lane 1					
HV	5.0	7.0	11.0	1.0	24.0
Total	5.0	7.0	11.0	1.0	24.0
Approach	5.0	7.0	11.0	1.0	24.0
From NORTH To:	NE	SE	S	NW	
Turn:	L3	L1	T1	R3	TOT
Lane 1					
LV	*	10.4	30.6	*	41.1
HV	1.0	0.6	1.6	*	3.2
Total	1.0	11.0	32.2	*	44.2
Lane 2					
LV	*	*	7.4	29.5	36.8
HV	*	*	0.4	1.5	1.9
Total	*	*	7.8	31.0	38.8
Approach	1.0	11.0	40.0	31.0	83.0
From NORTHWEST To:	N	NE	SE	S	
Turn:	L3	L2	T1	R1	TOT
Lane 1					
LV	12.4	*	155.6	*	168.0
HV	0.6	11.0	8.2	*	19.8
Total	13.0	11.0	163.8	*	187.8
Lane 2					
LV	*	*	29.6	135.9	165.5
HV	*	*	1.6	7.2	8.7
Total	*	*	31.2	143.0	174.2
Approach	13.0	11.0	195.0	143.0	362.0
* Movement not allocated to the lane					

## EXIT LANE FLOW RATES

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	80.0	11.2
Lane: 2	143.2	7.5
Total	223.3	18.8
Exit: SOUTHEAST		
Lane: 1	166.1	13.7
Lane: 2	58.1	3.1
Total	224.2	16.8
Exit: NORTHEAST		
Lane: 1	*	21.0
Total	*	21.0
Exit: NORTH		
Lane: 1	12.4	0.6
Lane: 2	14.2	1.8
Total	26.6	2.4
Exit: NORTHWEST		
Lane: 1	400.5	21.1

Lane: 2	140.1	18.4
Total	540.5	39.5

\* Movement not allocated to the lane

#### DOWNSTREAM LANE FLOW RATES FOR EXIT ROADS

Movement Class:	LV	HV
-----------------	----	----

Exit: SOUTH		
Lane: 1	80.0	11.2
Lane: 2	143.2	7.5
Total	223.3	18.8

Exit: SOUTHEAST		
Lane: 1	166.1	13.7
Lane: 2	58.1	3.1
Total	224.2	16.8

Exit: NORTHEAST		
Lane: 1	*	21.0
Total	*	21.0

Exit: NORTH		
Lane: 1	12.4	0.6
Lane: 2	14.2	1.8
Total	26.6	2.4

Exit: NORTHWEST		
Lane: 1	400.5	21.1
Lane: 2	140.1	18.4
Total	540.5	39.5

\* Movement not allocated to the lane

Unit Time for Volumes = 60 minutes

Peak Flow Period = 30 minutes

Flow Rates include effects of Flow Scale and Peak Flow Factor

[Go to Table Links \(Top\)](#)

## Other

### Model Settings Summary

Site:WD PM (NDR/Leeds) - existing

Intersection ID: 1  
Roundabout

\* Basic Parameters:  
Intersection Type: Roundabout  
Driving on the left-hand side of the road  
Input data specified in Metric units  
Model Defaults: New South Wales  
Peak Flow Period (for performance): 30 minutes  
Unit time (for volumes): 60 minutes.  
SIDRA Standard Delay model used  
SIDRA Standard Queue model used  
Level of Service based on: Delay (RTA NSW)  
Queue percentile: 95%

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### Diagnostics

Site:WD PM (NDR/Leeds) - existing

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SIDRA INTERSECTION 6.0.1.3703

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**SIDRA  
INTERSECTION 6**

**DETAILED OUTPUT****Site: Uni post PM**

New Site

Giveway / Yield (Two-Way)

**OUTPUT TABLE LINKS****Movements**

Intersection Negotiation Data  
 Gap Acceptance Parameters  
 Movement Capacity and Performance Parameters  
 Fuel Consumption, Emissions and Cost

**Lanes**

Lane Performance and Capacity Information  
 Lane Delays  
 Lane Queues  
 Lane Queue Percentiles  
 Lane Stops

**Flow Rates**

Origin-Destination Flow Rates (Total)  
 Origin-Destination Flow Rates by Movement Class  
 Lane Flow Rates

**Other**

Model Settings Summary  
 Diagnostics

**Movements**

Intersection Negotiation Data

Site:Uni post PM

Intersection ID: 1  
 Give-Way Sign Controlled Intersection

**INTERSECTION NEGOTIATION DATA**

From Approach	To Exit	Turn	Negn Radius m	Negn Speed km/h	Negn Dist. m	Appr. Dist. m	Downstream m	Distance User Spec?
South: Leeds Parade								
	North	T1	S	50.0	10.0	500	157	No
	East	R2	6.6	17.2	10.4	500	105	No
East: University								
	South	L2	10.0	20.2	15.7	500	108	No
	North	R2	6.6	17.2	10.4	500	103	No
North: Leeds Parade								
	East	L2	10.0	20.2	15.7	500	72	No
	South	T1	S	50.0	10.0	500	159	No

Downstream distance is distance travelled from the stopline until exit cruise speed is reached (includes negotiation distance). Acceleration distance is weighted for light and heavy vehicles. The same distance applies for both stopped and unstopped vehicles.

**MOVEMENT SPEEDS AND GEOMETRIC DELAY**

						Queue Move-up				
Mov ID	Turn	App. Speeds		Exit Speeds		1st Grn	2nd Grn	Av. Section Spd		Geom Delay sec
		Cruise	Negn	Negn	Cruise			Running	Overall	
South: Leeds Parade										
2	T1	50.0	50.0	50.0	50.0			50.0	50.0	0.0
3	R2	50.0	17.2	17.2	50.0	17.2		42.2	42.2	8.0
East: University										
4	L2	50.0	20.2	20.2	50.0	19.9		42.5	42.5	7.5
6	R2	50.0	17.2	17.2	50.0	19.9		42.5	42.5	8.0
North: Leeds Parade										



7	L2	50.0	20.2	20.2	50.0	49.8	49.8	7.5
8	T1	50.0	50.0	50.0	50.0	49.8	49.8	0.0

"Running Speed" is the average speed excluding stopped periods.

[Go to Table Links \(Top\)](#)

## Gap Acceptance Parameters

Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

Opd Lane	Dest	Opng Flow pcu/h	Critical Gap		Foll-up Headway sec	Entry HV Equiv	Intra Bunch Hdwy sec	Propn Bnchd
			Hdwy sec	Dist m				
South: Leeds Parade								
2	E	106	4.10	0.0	2.05	1.03	1.80	0.011
East: University								
1	S	104+	4.10	0.0	2.25	1.02	1.80	0.011
1	N	440+	5.43	0.0	3.07	1.02	1.11	0.045

North: Leeds Parade

No opposed movements on this approach.

Values in this table are adjusted for heavy vehicles in the entry stream.  
Use the Pedestrians and Priorities input dialogs to specify opposing pedestrian movements.  
+ Percentage of exiting flow included in opposing vehicle flow

[Go to Table Links \(Top\)](#)

## Movement Capacity and Performance Parameters

Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### MOVEMENT CAPACITY PARAMETERS

Mov ID	Turn Cl.	Mov Arv Flow veh/h	Opng Flow veh/h	Movement Adjust. Flow pcu/h	Total Cap. veh/h	Prac. Deg. Satn xp	Prac. Spare Cap. %	Deg. Satn x
South: Leeds Parade								
2	T1	#	301	0	0	1889	0.98	515
3	R2	#	26	0	0	1599	0.98	5927
East: University								
4	L2	#	78	0	0	1284	0.80	1217
6	R2	#	5	0	0	82	0.80	1217
North: Leeds Parade								
7	L2	#	2	0	0	37	0.98	1695
8	T1	#	101	0	0	1850	0.98	1695

\* Maximum degree of saturation

# Combined Movement Capacity parameters are shown for all Movement Classes.

### MOVEMENT PERFORMANCE

Mov ID	Turn	Total Delay (veh-h/h)	Total Delay (pers-h/h)	Aver. Delay (sec)	Eff. Stop Rate	Total Stops	Perf. Index	Tot.Trav. Distance (veh-km/h)	Tot.Trav. Time (veh-h/h)	Aver. Speed (km/h)
South: Leeds Parade										
2	T1	0.00	0.00	0.0	0.00	0.0	3.48	173.9	3.5	50.0
3	R2	0.00	0.00	8.3	0.61	16.0	0.45	15.1	0.4	42.2
East: University										
4	L2	0.01	0.01	8.0	0.60	46.8	1.54	45.5	1.1	42.5
6	R2	0.00	0.00	8.0	0.60	3.0	0.28	2.9	0.1	42.5
North: Leeds Parade										
7	L2	0.00	0.00	0.2	0.02	0.0	0.03	1.2	0.0	49.8
8	T1	0.00	0.00	0.2	0.02	2.2	1.18	58.4	1.2	49.8

[Go to Table Links \(Top\)](#)

## Fuel Consumption, Emissions and Cost

Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### FUEL CONSUMPTION, EMISSIONS AND COST (TOTAL)

Mov ID	Turn	Cost Total \$/h	Fuel Total L/h	CO2 Total kg/h	CO Total kg/h	HC Total kg/h	NOX Total kg/h
South: Leeds Parade							
2	T1	122.34	10.6	25.0	0.08	0.009	0.061
3	R2	12.52	1.0	2.4	0.01	0.001	0.006
		134.86	11.6	27.4	0.08	0.010	0.067
East: University							
4	L2	37.45	3.0	7.1	0.02	0.003	0.017
6	R2	2.40	0.2	0.5	0.00	0.000	0.001
		39.86	3.2	7.6	0.02	0.003	0.018
North: Leeds Parade							
7	L2	0.82	0.1	0.2	0.00	0.000	0.000
8	T1	41.18	3.6	8.4	0.03	0.003	0.021
		41.99	3.6	8.6	0.03	0.003	0.021
INTERSECTION:		216.71	18.4	43.6	0.13	0.016	0.106

### FUEL CONSUMPTION, EMISSIONS AND COST (RATE)

Mov ID	Turn	Cost Rate \$/km	Fuel Rate L/100km	CO2 Rate g/km	CO Rate g/km	HC Rate g/km	NOX Rate g/km
South: Leeds Parade							
2	T1	0.70	6.1	144.0	0.43	0.051	0.352
3	R2	0.83	6.7	157.5	0.49	0.062	0.382
		0.71	6.1	145.1	0.44	0.052	0.354
East: University							
4	L2	0.82	6.6	156.4	0.48	0.061	0.377
6	R2	0.82	6.6	156.4	0.48	0.061	0.377
		0.82	6.6	156.4	0.48	0.061	0.377
North: Leeds Parade							
7	L2	0.71	6.1	144.2	0.43	0.051	0.353
8	T1	0.71	6.1	144.2	0.43	0.051	0.353
		0.71	6.1	144.2	0.43	0.051	0.353
INTERSECTION:		0.61	5.2	122.3	0.37	0.045	0.298

[Go to Table Links \(Top\)](#)

## Lanes

### Lane Performance and Capacity Information

Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### LANE PERFORMANCE

						Q u e u e		
Lane	Flow	Cap	Deg.	Aver.	Eff.	95% Back		Lane
No.	veh/h	veh/h	Satn	Delay	Stop	-----		Length
			x	sec	Rate	veh	m	m
South: Leeds Parade								
1	301	1889	0.159	0.0	0.00			500.0
2	26	1599	0.016	8.3	0.61	0.1	0.5	60.0T

-----								
East: University								
1	83	1367	0.061	8.0	0.60	0.2	1.8	140.0
-----								
North: Leeds Parade								
1	103	1887	0.055	0.2	0.02			500.0
-----								
T Short lane due to specification of Turn Bay								

## LANE FLOW AND CAPACITY INFORMATION

-----					
Lane No.	Total Arv Flow (veh/h)	Min Cap veh/h	Tot Cap veh/h	Deg. Satn x	Lane Util %
-----					
South: Leeds Parade					
1	301	301	1889	0.159	100
2	26	6	1599	0.016	100
-----					
East: University					
1	83	6	1367	0.061	100
-----					
North: Leeds Parade					
1	103	103	1887	0.055	100
-----					

The capacity value for priority and continuous movements is obtained by adjusting the basic saturation flow for heavy vehicle and turning vehicle effects. Saturation flow scale applies if specified.

[Go to Table Links \(Top\)](#)

## Lane Delays

Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE DELAYS

			-----			Delay (seconds/veh)						-----	
Lane	Deg.	Prog.	Stop-line		Delay	Acc.	Queuing	Stopd					
No.	Satn	Factor	1st	2nd	Total	Dec.	Total	MvUp	(Idle)	Geom	Control		
	x		d1	d2	dSL	dn	dq	dqm	di	dig	dic		
-----													
South:	Leeds	Parade											
1	0.159			0.0					0.0	0.0			
2	0.016	1.000	0.3	0.0	0.3	0.7	0.0	0.0	0.0	8.0	8.3		
-----													
East:	University												
1	0.061	1.000	0.5	0.0	0.5	0.8	0.0	0.0	0.0	7.5	8.0		
-----													
North:	Leeds	Parade											
1	0.055			0.0					0.1	0.2			
-----													

SIDRA Standard Delay Model is used. Control Delay is the sum of Stop-line Delay and Geometric Delay.

dSL: Stop-line delay (=d1+d2)

dn: Average stop-start delay for all vehicles queued and unqueued

dq: Queuing delay (the part of the stop-line delay that includes stopped delay and queue move-up delay)

dqm: Queue move-up delay

di: Stopped delay (stopped (idling) time at near-zero speed)

dig: Geometric delay

dic: Control delay

## LANE DELAY PERCENTILES

-----								
Lane No.	Deg. Satn	Percentile Delay						
	x	50%	70%	85%	90%	95%	98%	100%
-----								
South:	Leeds Parade							
1	NA - Continuous Movement							
2	0.016	8.3	8.3	8.4	8.5	8.5	8.6	8.6
-----								
East:	University							
1	0.061	8.0	8.1	8.3	8.3	8.4	8.5	8.5
-----								
North:	Leeds Parade							
1	NA - Continuous Movement							
-----								

[Go to Table Links \(Top\)](#)

## Lane Queues

Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE QUEUES (VEHICLES)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (veh)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Nc	Queue 95%
	x		No	Nb1	Nb2	Nb	95%					
South: Leeds Parade												
2	0.016	1.000	0.0	0.0	0.0	0.0	0.1	0.00	0.0	100.0	0.0	0.0
East: University												
1	0.061	1.000	0.0	0.1	0.0	0.1	0.2	0.01	0.0	100.0	0.0	0.0
North: Leeds Parade												

## LANE QUEUES (DISTANCE)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (m)				Queue Stor.	Prob. Block	P'ile Block	Cyc-Av. Queue	
	x			Nb1	Nb2	Nb	95%	Ratio	%	%	Nc	95%
South: Leeds Parade												
2	0.016	1.000	0.0	0.2	0.0	0.2	0.5	0.00	0.0	100.0	0.0	0.0
East: University												
1	0.061	1.000	0.0	0.7	0.0	0.7	1.8	0.01	0.0	100.0	0.1	0.1
North: Leeds Parade												

[Go to Table Links \(Top\)](#)

## Lane Queue Percentiles

Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE QUEUE PERCENTILES (VEHICLES)

Lane No.	Deg. Satn x	Percentile Back of Queue (veh)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade								
2	0.016	0.0	0.0	0.1	0.1	0.1	0.1	0.1
East: University								
1	0.061	0.1	0.1	0.2	0.2	0.2	0.3	0.3
North: Leeds Parade								

## LANE QUEUE PERCENTILES (DISTANCE)

Lane No.	Deg. Satn x	Percentile Back of Queue (metres)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade								
2	0.016	0.2	0.3	0.4	0.4	0.5	0.6	0.6
East: University								
1	0.061	0.7	0.9	1.3	1.5	1.8	2.0	2.1
North: Leeds Parade								

[Go to Table Links \(Top\)](#)

## Lane Stops

### Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

	Deg.	Prog.	-- Effective Stop Rate --				Queue	Total		
Lane	Satn	Factor	Geom.		Overall	Total	Move-up	Queue	Prop.	
No.	x		he1	he2	hig	h	Stops	Rate	Move-ups	Queued
							H	hqm	Hqm	pq
South: Leeds Parade										
1	0.159	1.000			0.00	0.00	0.0			
2	0.016	1.000	0.07	0.00	0.54	0.61	16.0	0.00	0.0	0.21
East: University										
1	0.061	1.000	0.09	0.00	0.51	0.60	49.8	0.00	0.0	0.20
North: Leeds Parade										
1	0.055	1.000			0.02	0.02	2.2			
hig is the average value for all movements in a shared lane										
hqm is average queue move-up rate for all vehicles queued and unqueued										

[Go to Table Links \(Top\)](#)

## Flow Rates

### Origin-Destination Flow Rates (Total)

#### Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### TOTAL FLOW RATES (ALL MOVEMENT CLASSES)

From SOUTH To:		N	E	
Turn:		T1	R2	TOT
Flow Rate		301.0	26.0	327.0
%HV (all designations)		5.0	5.0	5.0
From EAST To:		S	N	
Turn:		L2	R2	TOT
Flow Rate		78.0	5.0	83.0
%HV (all designations)		5.0	5.0	5.0
From NORTH To:		E	S	
Turn:		L2	T1	TOT
Flow Rate		2.0	101.0	103.0
%HV (all designations)		5.0	5.0	5.0

[Go to Table Links \(Top\)](#)

### Origin-Destination Flow Rates by Movement Class

#### Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### FLOW RATES FOR Light Vehicles

From SOUTH To:		N	E	
Turn:		T1	R2	TOT
Flow Rate - Veh		286.0	24.7	310.7
Mov Class %		95.0	95.0	95.0
Flow Scale - Fixed		1.00	1.00	
Flow Scale - Var		1.00	1.00	
Peak Flow Factor		1.00	1.00	
From EAST To:		S	N	
Turn:		L2	R2	TOT
Flow Rate - Veh		74.1	4.8	78.8
Mov Class %		95.0	95.0	95.0
Flow Scale - Fixed		1.00	1.00	
Flow Scale - Var		1.00	1.00	
Peak Flow Factor		1.00	1.00	

From NORTH To:	E	S	
Turn:	L2	T1	TOT
Flow Rate - Veh	1.9	95.9	97.8
Mov Class %	95.0	95.0	95.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

## FLOW RATES FOR Heavy Vehicles

From SOUTH To:	N	E	
Turn:	T1	R2	TOT
Flow Rate - Veh	15.1	1.3	16.4
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

From EAST To:	S	N	
Turn:	L2	R2	TOT
Flow Rate - Veh	3.9	0.2	4.2
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

From NORTH To:	E	S	
Turn:	L2	T1	TOT
Flow Rate - Veh	0.1	5.1	5.2
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

[Go to Table Links \(Top\)](#)

## Lane Flow Rates

Site:Uni post PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE FLOW RATES AT STOP LINE

From SOUTH To:	N	E	
Turn:	T1	R2	TOT
Lane 1			
LV	286.0	*	286.0
HV	15.1	*	15.1
Total	301.0	*	301.0

Lane 2			
LV	*	24.7	24.7
HV	*	1.3	1.3
Total	*	26.0	26.0

Approach	301.0	26.0	327.0
----------	-------	------	-------

From EAST To:	S	N	
Turn:	L2	R2	TOT

Lane 1			
LV	74.1	4.8	78.8
HV	3.9	0.2	4.2
Total	78.0	5.0	83.0

Approach	78.0	5.0	83.0
----------	------	-----	------

From NORTH To:	E	S	
Turn:	L2	T1	TOT

Lane 1			
LV	1.9	95.9	97.8
HV	0.1	5.1	5.2
Total	2.0	101.0	103.0

Approach	2.0	101.0	103.0
----------	-----	-------	-------

\* Movement not allocated to the lane



## EXIT LANE FLOW RATES

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	170.0	9.0
Total	170.0	9.0
Exit: EAST		
Lane: 1	26.6	1.4
Total	26.6	1.4
Exit: NORTH		
Lane: 1	290.7	15.3
Total	290.7	15.3

\* Movement not allocated to the lane

## DOWNSTREAM LANE FLOW RATES FOR EXIT ROADS

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	170.0	9.0
Total	170.0	9.0
Exit: EAST		
Lane: 1	26.6	1.4
Total	26.6	1.4
Exit: NORTH		
Lane: 1	290.7	15.3
Total	290.7	15.3

\* Movement not allocated to the lane

Unit Time for Volumes = 60 minutes  
 Peak Flow Period = 30 minutes  
 Flow Rates include effects of Flow Scale and Peak Flow Factor

[Go to Table Links \(Top\)](#)

## Other

## Model Settings Summary

Site:Uni post PM

Intersection ID: 1  
 Give-Way Sign Controlled Intersection

\* Basic Parameters:  
 Intersection Type: Unsignalised - Give Way  
 Driving on the left-hand side of the road  
 Input data specified in Metric units  
 Model Defaults: New South Wales  
 Peak Flow Period (for performance): 30 minutes  
 Unit time (for volumes): 60 minutes.  
 SIDRA Standard Delay model used  
 SIDRA Standard Queue model used  
 Level of Service based on: Delay (RTA NSW)  
 Queue percentile: 95%

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## Diagnostics

Site:Uni post PM

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 SIDRA INTERSECTION 6.0.1.3703

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**SIDRA**  
**INTERSECTION 6**

## DETAILED OUTPUT

Site: Uni existing PM

New Site

Giveaway / Yield (Two-Way)

### OUTPUT TABLE LINKS



#### Movements

Intersection Negotiation Data  
Gap Acceptance Parameters  
Movement Capacity and Performance Parameters  
Fuel Consumption, Emissions and Cost



#### Lanes

Lane Performance and Capacity Information  
Lane Delays  
Lane Queues  
Lane Queue Percentiles  
Lane Stops



#### Flow Rates

Origin-Destination Flow Rates (Total)  
Origin-Destination Flow Rates by Movement Class  
Lane Flow Rates



#### Other

Model Settings Summary  
Diagnostics

### Movements

Intersection Negotiation Data

Site: Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### INTERSECTION NEGOTIATION DATA

From Approach	To Exit	Turn	Negn Radius m	Negn Speed km/h	Negn Dist. m	Appr. Dist. m	Downstream m	Distance User Spec?
South: Leeds Parade								
	North	T1	S	50.0	10.0	500	157	No
	East	R2	6.6	17.2	10.4	500	105	No
East: University								
	South	L2	10.0	20.2	15.7	140	108	No
	North	R2	6.6	17.2	10.4	140	102	No
North: Leeds Parade								
	East	L2	10.0	20.2	15.7	500	95	No
	South	T1	S	50.0	10.0	500	181	No

Downstream distance is distance travelled from the stopline until exit cruise speed is reached (includes negotiation distance). Acceleration distance is weighted for light and heavy vehicles. The same distance applies for both stopped and unstopped vehicles.

#### MOVEMENT SPEEDS AND GEOMETRIC DELAY

						Queue Move-up				
Mov	Turn	App. Speeds		Exit Speeds		1st	2nd	Av. Section Spd	Geom	
ID		Cruise	Negn	Negn	Cruise	Grn	Grn	Running Overall	Delay sec	
South: Leeds Parade										
2	T1	50.0	50.0	50.0	50.0			50.0	50.0	
3	R2	50.0	17.2	17.2	50.0	17.2		42.8	42.8	
East: University										
4	L2	50.0	20.2	20.2	50.0	20.1		35.3	35.3	
6	R2	50.0	17.2	17.2	50.0	20.1		35.3	35.3	
North: Leeds Parade										

7	L2	50.0	20.2	20.2	50.0	46.3	46.3	7.5
8	T1	50.0	50.0	50.0	50.0	46.3	46.3	0.0

"Running Speed" is the average speed excluding stopped periods.

[Go to Table Links \(Top\)](#)

## Gap Acceptance Parameters

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

Opd Lane	Dest	Opng Flow pcu/h	Critical Gap		Foll-up Headway sec	Entry HV Equiv	Intra Bunch Hdwy sec	Propn Bnchd
			Hdwy sec	Dist m				
South: Leeds Parade								
2	E	2	4.10	0.0	2.05	1.02	1.80	0.000
East: University								
1	S	1+	4.10	0.0	2.26	1.03	1.80	0.000
1	N	30+	5.43	0.0	3.07	1.02	1.43	0.004

North: Leeds Parade

No opposed movements on this approach.

Values in this table are adjusted for heavy vehicles in the entry stream.  
Use the Pedestrians and Priorities input dialogs to specify opposing pedestrian movements.  
+ Percentage of exiting flow included in opposing vehicle flow

[Go to Table Links \(Top\)](#)

## Movement Capacity and Performance Parameters

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### MOVEMENT CAPACITY PARAMETERS

Mov ID	Turn Cl.	Mov Arv Flow veh/h	Opng Flow veh/h	Movement Adjust. Flow pcu/h	Total Cap. veh/h	Prac. Deg. Satn xp	Prac. Spare Cap. %	Deg. Satn x
South: Leeds Parade								
2	T1	#	1	0	0	1889	0.98	**** 0.001
3	R2	#	27	0	0	1753	0.98	6263 0.015
East: University								
4	L2	#	82	0	0	1568	0.80	1430 0.052*
6	R2	#	1	0	0	19	0.80	1430 0.052*
North: Leeds Parade								
7	L2	#	1	0	0	920	0.98	**** 0.001
8	T1	#	1	0	0	920	0.98	**** 0.001

\* Maximum degree of saturation

# Combined Movement Capacity parameters are shown for all Movement Classes.

### MOVEMENT PERFORMANCE

Mov ID	Turn	Total Delay (veh-h/h)	Total Delay (pers-h/h)	Aver. Delay (sec)	Eff. Stop Rate	Total Stops	Perf. Index	Tot.Trav. Distance (veh-km/h)	Tot.Trav. Time (veh-h/h)	Aver. Speed (km/h)
South: Leeds Parade										
2	T1	0.00	0.00	0.0	0.00	0.0	0.01	0.6	0.0	50.0
3	R2	0.00	0.00	8.0	0.67	18.1	0.47	15.7	0.4	42.8
East: University										
4	L2	0.01	0.01	7.5	0.63	51.8	1.00	18.3	0.5	35.3
6	R2	0.00	0.00	7.5	0.63	0.6	0.18	0.2	0.0	35.3
North: Leeds Parade										
7	L2	0.00	0.00	3.7	0.40	0.4	0.02	0.6	0.0	46.3
8	T1	0.00	0.00	3.7	0.40	0.4	0.01	0.6	0.0	46.3

[Go to Table Links \(Top\)](#)

## Fuel Consumption, Emissions and Cost

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### FUEL CONSUMPTION, EMISSIONS AND COST (TOTAL)

Mov ID	Turn	Cost Total \$/h	Fuel Total L/h	CO2 Total kg/h	CO Total kg/h	HC Total kg/h	NOX Total kg/h
South: Leeds Parade							
2	T1	0.41	0.0	0.1	0.00	0.000	0.000
3	R2	12.84	1.0	2.5	0.01	0.001	0.006
		13.25	1.1	2.5	0.01	0.001	0.006
East: University							
4	L2	19.43	1.7	3.9	0.01	0.001	0.012
6	R2	0.24	0.0	0.0	0.00	0.000	0.000
		19.67	1.7	4.0	0.01	0.001	0.012
North: Leeds Parade							
7	L2	0.44	0.0	0.1	0.00	0.000	0.000
8	T1	0.44	0.0	0.1	0.00	0.000	0.000
		0.88	0.1	0.2	0.00	0.000	0.000
INTERSECTION:		33.80	2.8	6.7	0.02	0.003	0.019

### FUEL CONSUMPTION, EMISSIONS AND COST (RATE)

Mov ID	Turn	Cost Rate \$/km	Fuel Rate L/100km	CO2 Rate g/km	CO Rate g/km	HC Rate g/km	NOX Rate g/km
South: Leeds Parade							
2	T1	0.70	6.1	144.0	0.43	0.051	0.352
3	R2	0.82	6.6	156.5	0.48	0.061	0.381
		0.82	6.6	156.0	0.48	0.061	0.380
East: University							
4	L2	1.06	9.0	213.5	0.60	0.079	0.669
6	R2	1.06	9.0	213.5	0.60	0.079	0.669
		1.06	9.0	213.5	0.60	0.079	0.669
North: Leeds Parade							
7	L2	0.76	6.3	149.5	0.46	0.056	0.364
8	T1	0.76	6.3	149.5	0.46	0.056	0.364
		0.76	6.3	149.5	0.46	0.056	0.364
INTERSECTION:		0.78	6.5	154.5	0.45	0.058	0.440

[Go to Table Links \(Top\)](#)

## Lanes

### Lane Performance and Capacity Information

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### LANE PERFORMANCE

						Q u e u e		
Lane	Flow	Cap	Deg.	Aver.	Eff.	95% Back		Lane
No.	veh/h	veh/h	Satn	Delay	Stop	-----		Length
			x	sec	Rate	veh	m	m
South: Leeds Parade								
1	1	1889	0.001	0.0	0.00			500.0
2	27	1753	0.015	8.0	0.67	0.1	0.5	60.0T

-----								
East: University								
1	83	1587	0.052	7.5	0.63	0.2	1.6	140.0
-----								
North: Leeds Parade								
1	2	1840	0.001	3.7	0.40			500.0
-----								
T Short lane due to specification of Turn Bay								

## LANE FLOW AND CAPACITY INFORMATION

-----					
Lane No.	Total Arv Flow (veh/h)	Min Cap veh/h	Tot Cap veh/h	Deg. Satn x	Lane Util %
-----					
South: Leeds Parade					
1	1	1	1889	0.001	100
2	27	6	1753	0.015	100
-----					
East: University					
1	83	6	1587	0.052	100
-----					
North: Leeds Parade					
1	2	2	1840	0.001	100
-----					

The capacity value for priority and continuous movements is obtained by adjusting the basic saturation flow for heavy vehicle and turning vehicle effects. Saturation flow scale applies if specified.

[Go to Table Links \(Top\)](#)

## Lane Delays

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE DELAYS

						Delay (seconds/veh)					
Lane No.	Deg. Satn	Prog. Factor	Stop 1st	line 2nd	Delay Total	Acc. Dec. dn	Queuing Total dq	MvUp dqm	Stopd (Idle) di	Geom dig	Control dic
	x		d1	d2	dSL						
-----											
South: 1	Leeds 0.001	Parade		0.0					0.0	0.0	
2	0.015	1.000	0.0	0.0	0.0	0.1	0.0	0.0	0.0	8.0	8.0
-----											
East: 1	University 0.052		0.0	0.0	0.0	0.0	0.0	0.0	0.0	7.5	7.5
-----											
North: 1	Leeds 0.001	Parade		0.0					3.7	3.7	
-----											

SIDRA Standard Delay Model is used. Control Delay is the sum of Stop-line Delay and Geometric Delay.

dSL: Stop-line delay (=d1+d2)

dn: Average stop-start delay for all vehicles queued and unqueued

dq: Queuing delay (the part of the stop-line delay that includes stopped delay and queue move-up delay)

dqm: Queue move-up delay

di: Stopped delay (stopped (idling) time at near-zero speed)

dig: Geometric delay

dic: Control delay

## LANE DELAY PERCENTILES

-----								
Lane No.	Deg. Satn x	Percentile Delay						
		50%	70%	85%	90%	95%	98%	100%
-----								
South: Leeds Parade								
1	NA - Continuous Movement							
2	0.015	8.0	8.0	8.0	8.0	8.0	8.0	8.0
-----								
East: University								
1	0.052	7.5	7.5	7.5	7.5	7.5	7.5	7.5
-----								
North: Leeds Parade								
1	NA - Continuous Movement							
-----								

[Go to Table Links \(Top\)](#)

## Lane Queues

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE QUEUES (VEHICLES)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (veh)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Nc	Queue 95%
	x		No	Nb1	Nb2	Nb	95%					
South: Leeds Parade												
2	0.015	1.000	0.0	0.0	0.0	0.0	0.1	0.00	0.0	100.0	0.0	0.0
East: University												
1	0.052	1.000	0.0	0.1	0.0	0.1	0.2	0.00	0.0	100.0	0.0	0.0
North: Leeds Parade												

## LANE QUEUES (DISTANCE)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (m)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Queue	
	x			Nb1	Nb2	Nb	95%				Nc	95%
South: Leeds Parade												
2	0.015	1.000	0.0	0.2	0.0	0.2	0.5	0.00	0.0	100.0	0.0	0.0
East: University												
1	0.052	1.000	0.0	0.6	0.0	0.6	1.6	0.00	0.0	100.0	0.0	0.0
North: Leeds Parade												

[Go to Table Links \(Top\)](#)

## Lane Queue Percentiles

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE QUEUE PERCENTILES (VEHICLES)

Lane No.	Deg. Satn x	Percentile Back of Queue (veh)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade								
2	0.015	0.0	0.0	0.1	0.1	0.1	0.1	0.1
East: University								
1	0.052	0.1	0.1	0.2	0.2	0.2	0.2	0.3
North: Leeds Parade								

## LANE QUEUE PERCENTILES (DISTANCE)

Lane No.	Deg. Satn x	Percentile Back of Queue (metres)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade								
2	0.015	0.2	0.3	0.4	0.4	0.5	0.6	0.6
East: University								
1	0.052	0.6	0.8	1.2	1.3	1.6	1.7	1.9
North: Leeds Parade								

[Go to Table Links \(Top\)](#)



## Lane Stops

### Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

	Deg.	Prog.	-- Effective Stop Rate --				Queue	Total		
Lane	Satn	Factor	Geom.		Overall	Total	Move-up	Queue	Prop.	
No.	x		he1	he2	hig	h	Stops	Rate	Move-ups	Queued
							H	hqm	Hqm	pq
South: Leeds Parade										
1	0.001	1.000			0.00	0.00	0.0			
2	0.015	1.000	0.00	0.00	0.67	0.67	18.1	0.00	0.0	0.02
East: University										
1	0.052	1.000	0.00	0.00	0.63	0.63	52.4	0.00	0.0	0.01
North: Leeds Parade										
1	0.001	1.000			0.40	0.40	0.8			
hig is the average value for all movements in a shared lane										
hqm is average queue move-up rate for all vehicles queued and unqueued										

[Go to Table Links \(Top\)](#)

## Flow Rates

### Origin-Destination Flow Rates (Total)

#### Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### TOTAL FLOW RATES (ALL MOVEMENT CLASSES)

From SOUTH To:		N	E	
Turn:		T1	R2	TOT
Flow Rate		1.0	27.0	28.0
%HV (all designations)		5.0	5.0	5.0
From EAST To:		S	N	
Turn:		L2	R2	TOT
Flow Rate		82.0	1.0	83.0
%HV (all designations)		5.0	5.0	5.0
From NORTH To:		E	S	
Turn:		L2	T1	TOT
Flow Rate		1.0	1.0	2.0
%HV (all designations)		5.0	5.0	5.0

[Go to Table Links \(Top\)](#)

### Origin-Destination Flow Rates by Movement Class

#### Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### FLOW RATES FOR Light Vehicles

From SOUTH To:		N	E	
Turn:		T1	R2	TOT
Flow Rate - Veh		0.9	25.6	26.6
Mov Class %		95.0	95.0	95.0
Flow Scale - Fixed		1.00	1.00	
Flow Scale - Var		1.00	1.00	
Peak Flow Factor		1.00	1.00	
From EAST To:		S	N	
Turn:		L2	R2	TOT
Flow Rate - Veh		77.9	0.9	78.8
Mov Class %		95.0	95.0	95.0
Flow Scale - Fixed		1.00	1.00	
Flow Scale - Var		1.00	1.00	
Peak Flow Factor		1.00	1.00	

From NORTH To:	E	S	
Turn:	L2	T1	TOT
Flow Rate - Veh	0.9	0.9	1.9
Mov Class %	95.0	95.0	95.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

## FLOW RATES FOR Heavy Vehicles

From SOUTH To:	N	E	
Turn:	T1	R2	TOT
Flow Rate - Veh	0.1	1.4	1.4
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

From EAST To:	S	N	
Turn:	L2	R2	TOT
Flow Rate - Veh	4.1	0.1	4.2
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

From NORTH To:	E	S	
Turn:	L2	T1	TOT
Flow Rate - Veh	0.1	0.1	0.1
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

[Go to Table Links \(Top\)](#)

## Lane Flow Rates

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE FLOW RATES AT STOP LINE

From SOUTH To:	N	E	
Turn:	T1	R2	TOT
Lane 1			
LV	0.9	*	0.9
HV	0.1	*	0.1
Total	1.0	*	1.0

Lane 2			
LV	*	25.6	25.6
HV	*	1.4	1.4
Total	*	27.0	27.0

Approach	1.0	27.0	28.0
----------	-----	------	------

From EAST To:	S	N	
Turn:	L2	R2	TOT

Lane 1			
LV	77.9	0.9	78.8
HV	4.1	0.1	4.2
Total	82.0	1.0	83.0

Approach	82.0	1.0	83.0
----------	------	-----	------

From NORTH To:	E	S	
Turn:	L2	T1	TOT

Lane 1			
LV	0.9	0.9	1.9
HV	0.1	0.1	0.1
Total	1.0	1.0	2.0

Approach	1.0	1.0	2.0
----------	-----	-----	-----

\* Movement not allocated to the lane

## EXIT LANE FLOW RATES

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	78.8	4.2
Total	78.8	4.2
Exit: EAST		
Lane: 1	26.6	1.4
Total	26.6	1.4
Exit: NORTH		
Lane: 1	1.9	0.1
Total	1.9	0.1

\* Movement not allocated to the lane

## DOWNSTREAM LANE FLOW RATES FOR EXIT ROADS

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	78.8	4.2
Total	78.8	4.2
Exit: EAST		
Lane: 1	26.6	1.4
Total	26.6	1.4
Exit: NORTH		
Lane: 1	1.9	0.1
Total	1.9	0.1

\* Movement not allocated to the lane

Unit Time for Volumes = 60 minutes  
Peak Flow Period = 30 minutes  
Flow Rates include effects of Flow Scale and Peak Flow Factor

[Go to Table Links \(Top\)](#)

## Other

## Model Settings Summary

Site:Uni existing PM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

\* Basic Parameters:  
Intersection Type: Unsignalised - Give Way  
Driving on the left-hand side of the road  
Input data specified in Metric units  
Model Defaults: New South Wales  
Peak Flow Period (for performance): 30 minutes  
Unit time (for volumes): 60 minutes.  
SIDRA Standard Delay model used  
SIDRA Standard Queue model used  
Level of Service based on: Delay (RTA NSW)  
Queue percentile: 95%

[Go to Table Links \(Top\)](#)

## Diagnostics

Site:Uni existing PM

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**SIDRA**  
**INTERSECTION 6**

**DETAILED OUTPUT****Site: Uni post AM**

New Site

Giveaway / Yield (Two-Way)

**OUTPUT TABLE LINKS****Movements**

Intersection Negotiation Data  
 Gap Acceptance Parameters  
 Movement Capacity and Performance Parameters  
 Fuel Consumption, Emissions and Cost

**Lanes**

Lane Performance and Capacity Information  
 Lane Delays  
 Lane Queues  
 Lane Queue Percentiles  
 Lane Stops

**Flow Rates**

Origin-Destination Flow Rates (Total)  
 Origin-Destination Flow Rates by Movement Class  
 Lane Flow Rates

**Other**

Model Settings Summary  
 Diagnostics

**Movements**

Intersection Negotiation Data

Site:Uni post AM

Intersection ID: 1  
 Give-Way Sign Controlled Intersection

**INTERSECTION NEGOTIATION DATA**

From Approach	To Exit	Turn	Negn Radius m	Negn Speed km/h	Negn Dist. m	Appr. Dist. m	Downstream m	Distance User Spec?
South: Leeds Parade								
	North	T1	S	50.0	10.0	500	157	No
	East	R2	6.6	17.2	10.4	500	105	No
East: University								
	South	L2	10.0	20.2	15.7	500	108	No
	North	R2	6.6	17.2	10.4	500	103	No
North: Leeds Parade								
	East	L2	10.0	20.2	15.7	500	72	No
	South	T1	S	50.0	10.0	500	158	No

Downstream distance is distance travelled from the stopline until exit cruise speed is reached (includes negotiation distance). Acceleration distance is weighted for light and heavy vehicles. The same distance applies for both stopped and unstopped vehicles.

**MOVEMENT SPEEDS AND GEOMETRIC DELAY**

						Queue Move-up				
Mov ID	Turn	App. Speeds		Exit Speeds		1st Grn	2nd Grn	Av. Section Spd		Geom Delay sec
		Cruise	Negn	Negn	Cruise			Running	Overall	
South: Leeds Parade										
2	T1	50.0	50.0	50.0	50.0			50.0	50.0	0.0
3	R2	50.0	17.2	17.2	50.0	17.2		41.6	41.6	8.0
East: University										
4	L2	50.0	20.2	20.2	50.0	19.7		41.8	41.8	7.5
6	R2	50.0	17.2	17.2	50.0	19.7		41.8	41.8	8.0
North: Leeds Parade										

7	L2	50.0	20.2	20.2	50.0	49.8	49.8	7.5
8	T1	50.0	50.0	50.0	50.0	49.8	49.8	0.0

"Running Speed" is the average speed excluding stopped periods.

[Go to Table Links \(Top\)](#)

## Gap Acceptance Parameters

Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

Opd Lane	Dest	Opng Flow pcu/h	Critical Gap		Foll-up Headway sec	Entry HV Equiv	Intra Bunch Hdwy sec	Propn Bnchd
			Hdwy sec	Dist m				
South: Leeds Parade								
2	E	314	4.10	0.0	2.05	1.02	1.80	0.036
East: University								
1	S	309+	4.10	0.0	2.26	1.03	1.80	0.035
1	N	495+	5.43	0.0	3.07	1.02	0.98	0.045

North: Leeds Parade

No opposed movements on this approach.

Values in this table are adjusted for heavy vehicles in the entry stream.  
Use the Pedestrians and Priorities input dialogs to specify opposing pedestrian movements.  
+ Percentage of exiting flow included in opposing vehicle flow

[Go to Table Links \(Top\)](#)

## Movement Capacity and Performance Parameters

Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### MOVEMENT CAPACITY PARAMETERS

Mov ID	Turn Cl.	Mov Arv	Opng Flow veh/h	Movement Adjust. Flow pcu/h	Total Cap. veh/h	Prac. Deg. Satn xp	Prac. Spare Cap. %	Deg. Satn x
South: Leeds Parade								
2	T1	#	101	0	0	1889	0.98	1733
3	R2	#	78	0	0	1298	0.98	1530
East: University								
4	L2	#	26	0	0	1046	0.80	3118
6	R2	#	2	0	0	80	0.80	3118
North: Leeds Parade								
7	L2	#	5	0	0	31	0.98	504
8	T1	#	301	0	0	1856	0.98	504

\* Maximum degree of saturation

# Combined Movement Capacity parameters are shown for all Movement Classes.

### MOVEMENT PERFORMANCE

Mov ID	Turn	Total Delay (veh-h/h)	Total Delay (pers-h/h)	Aver. Delay (sec)	Eff. Stop Rate	Total Stops	Perf. Index	Tot.Trav. Distance (veh-km/h)	Tot.Trav. Time (veh-h/h)	Aver. Speed (km/h)
South: Leeds Parade										
2	T1	0.00	0.00	0.0	0.00	0.0	1.17	58.4	1.2	50.0
3	R2	0.01	0.01	9.1	0.66	51.7	1.39	45.2	1.1	41.6
East: University										
4	L2	0.00	0.00	8.7	0.63	16.3	0.53	15.2	0.4	41.8
6	R2	0.00	0.00	8.7	0.63	1.3	0.11	1.2	0.0	41.8
North: Leeds Parade										
7	L2	0.00	0.00	0.1	0.02	0.1	0.07	2.9	0.1	49.8
8	T1	0.00	0.00	0.1	0.02	5.5	3.51	173.9	3.5	49.8

[Go to Table Links \(Top\)](#)

## Fuel Consumption, Emissions and Cost

Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### FUEL CONSUMPTION, EMISSIONS AND COST (TOTAL)

Mov ID	Turn	Cost Total \$/h	Fuel Total L/h	CO2 Total kg/h	CO Total kg/h	HC Total kg/h	NOX Total kg/h
South: Leeds Parade							
2	T1	41.05	3.6	8.4	0.03	0.003	0.021
3	R2	38.03	3.0	7.2	0.02	0.003	0.017
		79.08	6.6	15.6	0.05	0.006	0.038
East: University							
4	L2	12.65	1.0	2.4	0.01	0.001	0.006
6	R2	0.97	0.1	0.2	0.00	0.000	0.000
		13.62	1.1	2.6	0.01	0.001	0.006
North: Leeds Parade							
7	L2	2.04	0.2	0.4	0.00	0.000	0.001
8	T1	122.67	10.6	25.1	0.08	0.009	0.061
		124.71	10.8	25.5	0.08	0.009	0.062
INTERSECTION:		217.40	18.5	43.6	0.13	0.016	0.106

### FUEL CONSUMPTION, EMISSIONS AND COST (RATE)

Mov ID	Turn	Cost Rate \$/km	Fuel Rate L/100km	CO2 Rate g/km	CO Rate g/km	HC Rate g/km	NOX Rate g/km
South: Leeds Parade							
2	T1	0.70	6.1	144.0	0.43	0.051	0.352
3	R2	0.84	6.7	158.5	0.49	0.063	0.383
		0.76	6.4	150.3	0.46	0.056	0.366
East: University							
4	L2	0.83	6.7	157.5	0.49	0.062	0.379
6	R2	0.83	6.7	157.5	0.49	0.062	0.379
		0.83	6.7	157.5	0.49	0.062	0.379
North: Leeds Parade							
7	L2	0.71	6.1	144.2	0.43	0.051	0.352
8	T1	0.71	6.1	144.2	0.43	0.051	0.352
		0.71	6.1	144.2	0.43	0.051	0.352
INTERSECTION:		0.61	5.2	122.5	0.37	0.045	0.299

[Go to Table Links \(Top\)](#)

## Lanes

## Lane Performance and Capacity Information

Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### LANE PERFORMANCE

						Q u e u e		
Lane	Flow	Cap	Deg.	Aver.	Eff.	95% Back		Lane
No.	veh/h	veh/h	Satn x	Delay sec	Stop Rate	----- veh m		Length m
South: Leeds Parade								
1	101	1889	0.053	0.0	0.00			500.0
2	78	1298	0.060	9.1	0.66	0.3	1.9	60.0T



-----								
East: University								
1	28	1126	0.025	8.7	0.63	0.1	0.7	140.0
-----								
North: Leeds Parade								
1	306	1887	0.162	0.1	0.02			500.0
-----								
T Short lane due to specification of Turn Bay								

## LANE FLOW AND CAPACITY INFORMATION

-----					
Lane No.	Total Arv Flow (veh/h)	Min Cap veh/h	Tot Cap veh/h	Deg. Satn x	Lane Util %
-----					
South: Leeds Parade					
1	101	101	1889	0.053	100
2	78	6	1298	0.060	100
-----					
East: University					
1	28	6	1126	0.025	100
-----					
North: Leeds Parade					
1	306	306	1887	0.162	100
-----					

The capacity value for priority and continuous movements is obtained by adjusting the basic saturation flow for heavy vehicle and turning vehicle effects. Saturation flow scale applies if specified.

[Go to Table Links \(Top\)](#)

## Lane Delays

Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE DELAYS

-----											
			Delay (seconds/veh)								
Lane No.	Deg. Satn x	Prog. Factor	Stop-line Delay			Acc. Dec. dn	Queuing		Stopd (Idle) di	Geom dig	Control dic
			1st d1	2nd d2	Total dSL		Total dq	MvUp dqm			
-----											
South: Leeds Parade											
1	0.053			0.0					0.0	0.0	
2	0.060	1.000	1.1	0.0	1.1	1.4	0.0	0.0	0.0	8.0	9.1
-----											
East: University											
1	0.025	1.000	1.2	0.0	1.2	1.5	0.0	0.0	0.0	7.5	8.7
-----											
North: Leeds Parade											
1	0.162			0.0					0.1	0.1	
-----											

SIDRA Standard Delay Model is used. Control Delay is the sum of Stop-line Delay and Geometric Delay.

dSL: Stop-line delay (=d1+d2)

dn: Average stop-start delay for all vehicles queued and unqueued

dq: Queuing delay (the part of the stop-line delay that includes stopped delay and queue move-up delay)

dqm: Queue move-up delay

di: Stopped delay (stopped (idling) time at near-zero speed)

dig: Geometric delay

dic: Control delay

## LANE DELAY PERCENTILES

-----								
Lane No.	Deg. Satn x	Percentile Delay						
		50%	70%	85%	90%	95%	98%	100%
-----								
South: Leeds Parade								
1	NA - Continuous Movement							
2	0.060	9.1	9.3	9.6	9.8	10.0	10.1	10.2
-----								
East: University								
1	0.025	8.7	9.0	9.3	9.5	9.7	9.9	10.0
-----								
North: Leeds Parade								
1	NA - Continuous Movement							
-----								

[Go to Table Links \(Top\)](#)

## Lane Queues

### Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### LANE QUEUES (VEHICLES)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (veh)				Queue Stor.	Prob. Block	P'ile Block	Cyc-Av.	Queue
	x		No	Nb1	Nb2	Nb	95%	Ratio	%	%	Nc	95%
South: Leeds Parade												
2	0.060	1.000	0.0	0.1	0.0	0.1	0.3	0.01	0.0	100.0	0.0	0.0
East: University												
1	0.025	1.000	0.0	0.0	0.0	0.0	0.1	0.00	0.0	100.0	0.0	0.0
North: Leeds Parade												

#### LANE QUEUES (DISTANCE)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue	Back of Queue (m)				Queue Stor.	Prob. Block	P'ile Block	Cyc-Av. Queue	
	x		No	Nb1	Nb2	Nb	95%	Ratio	%	%	Nc	95%
South: Leeds Parade												
2	0.060	1.000	0.0	0.8	0.0	0.8	1.9	0.01	0.0	100.0	0.2	0.3
East: University												
1	0.025	1.000	0.0	0.3	0.0	0.3	0.7	0.00	0.0	100.0	0.1	0.1
North: Leeds Parade												

[Go to Table Links \(Top\)](#)

## Lane Queue Percentiles

### Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### LANE QUEUE PERCENTILES (VEHICLES)

Lane No.	Deg. Satn x	Percentile Back of Queue (veh)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade								
2	0.060	0.1	0.1	0.2	0.2	0.3	0.3	0.3
East: University								
1	0.025	0.0	0.0	0.1	0.1	0.1	0.1	0.1
North: Leeds Parade								

#### LANE QUEUE PERCENTILES (DISTANCE)

Lane No.	Deg. Satn x	Percentile Back of Queue (metres)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade								
2	0.060	0.8	1.0	1.4	1.6	1.9	2.1	2.3
East: University								
1	0.025	0.3	0.4	0.5	0.6	0.7	0.8	0.8
North: Leeds Parade								

[Go to Table Links \(Top\)](#)

## Lane Stops

### Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

	Deg.	Prog.	-- Effective Stop				Queue	Total		
Lane	Satn	Factor	Geom.		Rate	--	Total	Move-up	Queue	Prop.
No.	x		he1	he2	hig	h	Stops	Rate	Move-ups	Queued
							H	hqm	Hqm	pq
South: Leeds Parade										
1	0.053	1.000			0.00	0.00	0.0			
2	0.060	1.000	0.25	0.00	0.41	0.66	51.7	0.00	0.0	0.40
East: University										
1	0.025	1.000	0.23	0.00	0.40	0.63	17.6	0.00	0.0	0.38
North: Leeds Parade										
1	0.162	1.000			0.02	0.02	5.6			
hig is the average value for all movements in a shared lane										
hqm is average queue move-up rate for all vehicles queued and unqueued										

[Go to Table Links \(Top\)](#)

## Flow Rates

### Origin-Destination Flow Rates (Total)

#### Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### TOTAL FLOW RATES (ALL MOVEMENT CLASSES)

From SOUTH To:		N	E	
Turn:		T1	R2	TOT
Flow Rate		101.0	78.0	179.0
%HV (all designations)		5.0	5.0	5.0
From EAST To:		S	N	
Turn:		L2	R2	TOT
Flow Rate		26.0	2.0	28.0
%HV (all designations)		5.0	5.0	5.0
From NORTH To:		E	S	
Turn:		L2	T1	TOT
Flow Rate		5.0	301.0	306.0
%HV (all designations)		5.0	5.0	5.0

[Go to Table Links \(Top\)](#)

### Origin-Destination Flow Rates by Movement Class

#### Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### FLOW RATES FOR Light Vehicles

From SOUTH To:		N	E	
Turn:		T1	R2	TOT
Flow Rate - Veh		95.9	74.1	170.0
Mov Class %		95.0	95.0	95.0
Flow Scale - Fixed		1.00	1.00	
Flow Scale - Var		1.00	1.00	
Peak Flow Factor		1.00	1.00	
From EAST To:		S	N	
Turn:		L2	R2	TOT
Flow Rate - Veh		24.7	1.9	26.6
Mov Class %		95.0	95.0	95.0
Flow Scale - Fixed		1.00	1.00	
Flow Scale - Var		1.00	1.00	
Peak Flow Factor		1.00	1.00	

From NORTH To:	E	S	
Turn:	L2	T1	TOT
Flow Rate - Veh	4.8	286.0	290.7
Mov Class %	95.0	95.0	95.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

## FLOW RATES FOR Heavy Vehicles

From SOUTH To:	N	E	
Turn:	T1	R2	TOT
Flow Rate - Veh	5.1	3.9	9.0
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

From EAST To:	S	N	
Turn:	L2	R2	TOT
Flow Rate - Veh	1.3	0.1	1.4
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

From NORTH To:	E	S	
Turn:	L2	T1	TOT
Flow Rate - Veh	0.2	15.1	15.3
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

[Go to Table Links \(Top\)](#)

## Lane Flow Rates

Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE FLOW RATES AT STOP LINE

From SOUTH To:	N	E	
Turn:	T1	R2	TOT
Lane 1			
LV	95.9	*	95.9
HV	5.1	*	5.1
Total	101.0	*	101.0

Lane 2			
LV	*	74.1	74.1
HV	*	3.9	3.9
Total	*	78.0	78.0

Approach	101.0	78.0	179.0
----------	-------	------	-------

From EAST To:	S	N	
Turn:	L2	R2	TOT

Lane 1			
LV	24.7	1.9	26.6
HV	1.3	0.1	1.4
Total	26.0	2.0	28.0

Approach	26.0	2.0	28.0
----------	------	-----	------

From NORTH To:	E	S	
Turn:	L2	T1	TOT

Lane 1			
LV	4.8	286.0	290.7
HV	0.2	15.1	15.3
Total	5.0	301.0	306.0

Approach	5.0	301.0	306.0
----------	-----	-------	-------

\* Movement not allocated to the lane

## EXIT LANE FLOW RATES

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	310.7	16.4
Total	310.7	16.4
Exit: EAST		
Lane: 1	78.8	4.2
Total	78.8	4.2
Exit: NORTH		
Lane: 1	97.8	5.2
Total	97.8	5.2

\* Movement not allocated to the lane

## DOWNSTREAM LANE FLOW RATES FOR EXIT ROADS

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	310.7	16.4
Total	310.7	16.4
Exit: EAST		
Lane: 1	78.8	4.2
Total	78.8	4.2
Exit: NORTH		
Lane: 1	97.8	5.2
Total	97.8	5.2

\* Movement not allocated to the lane

Unit Time for Volumes = 60 minutes

Peak Flow Period = 30 minutes

Flow Rates include effects of Flow Scale and Peak Flow Factor

[Go to Table Links \(Top\)](#)

## Other

## Model Settings Summary

Site:Uni post AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

\* Basic Parameters:  
Intersection Type: Unsignalised - Give Way  
Driving on the left-hand side of the road  
Input data specified in Metric units  
Model Defaults: New South Wales  
Peak Flow Period (for performance): 30 minutes  
Unit time (for volumes): 60 minutes.  
SIDRA Standard Delay model used  
SIDRA Standard Queue model used  
Level of Service based on: Delay (RTA NSW)  
Queue percentile: 95%

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## Diagnostics

Site:Uni post AM

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**SIDRA**  
**INTERSECTION 6**

## DETAILED OUTPUT

Site: Uni existing AM

New Site

Giveway / Yield (Two-Way)

### OUTPUT TABLE LINKS



#### Movements

Intersection Negotiation Data  
Gap Acceptance Parameters  
Movement Capacity and Performance Parameters  
Fuel Consumption, Emissions and Cost



#### Lanes

Lane Performance and Capacity Information  
Lane Delays  
Lane Queues  
Lane Queue Percentiles  
Lane Stops



#### Flow Rates

Origin-Destination Flow Rates (Total)  
Origin-Destination Flow Rates by Movement Class  
Lane Flow Rates



#### Other

Model Settings Summary  
Diagnostics

### Movements

Intersection Negotiation Data

Site: Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### INTERSECTION NEGOTIATION DATA

From Approach	To Exit	Turn	Negn Radius m	Negn Speed km/h	Negn Dist. m	Appr. Dist. m	Downstream m	Distance User Spec?
South: Leeds Parade								
	North	T1	S	50.0	10.0	500	157	No
	East	R2	6.6	17.2	10.4	500	105	No
East: University								
	South	L2	10.0	20.2	15.7	500	108	No
	North	R2	6.6	17.2	10.4	500	102	No
North: Leeds Parade								
	East	L2	10.0	20.2	15.7	500	95	No
	South	T1	S	50.0	10.0	500	181	No

Downstream distance is distance travelled from the stopline until exit cruise speed is reached (includes negotiation distance). Acceleration distance is weighted for light and heavy vehicles. The same distance applies for both stopped and unstopped vehicles.

#### MOVEMENT SPEEDS AND GEOMETRIC DELAY

Mov ID	Turn	App. Speeds		Exit Speeds		Queue Move-up		Av. Section Spd		Geom Delay sec
		Cruise	Negn	Negn	Cruise	1st	2nd	Running	Overall	
						Grn	Grn			
South: Leeds Parade										
2	T1	50.0	50.0	50.0	50.0			50.0	50.0	0.0
3	R2	50.0	17.2	17.2	50.0	17.2		42.7	42.7	8.0
East: University										
4	L2	50.0	20.2	20.2	50.0	20.1		43.1	43.1	7.5
6	R2	50.0	17.2	17.2	50.0	20.1		43.1	43.1	8.0
North: Leeds Parade										



7	L2	50.0	20.2	20.2	50.0	46.3	46.3	7.5
8	T1	50.0	50.0	50.0	50.0	46.3	46.3	0.0

"Running Speed" is the average speed excluding stopped periods.

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## Gap Acceptance Parameters

Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

Opd Lane	Dest	Opng Flow pcu/h	Critical Gap		Foll-up Headway sec	Entry HV Equiv	Intra Bunch Hdwy sec	Propn Bnchd
			Hdwy sec	Dist m				
South: Leeds Parade								
2	E	2	4.10	0.0	2.05	1.03	1.80	0.000
East: University								
1	S	1+	4.10	0.0	2.25	1.02	1.80	0.000
1	N	87+	5.43	0.0	3.07	1.02	1.57	0.012

North: Leeds Parade

No opposed movements on this approach.

Values in this table are adjusted for heavy vehicles in the entry stream.  
Use the Pedestrians and Priorities input dialogs to specify opposing pedestrian movements.  
+ Percentage of exiting flow included in opposing vehicle flow

[Go to Table Links \(Top\)](#)

## Movement Capacity and Performance Parameters

Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### MOVEMENT CAPACITY PARAMETERS

Mov ID	Turn Cl.	Mov Arv Flow veh/h	Opng Flow veh/h	Movement Adjust. Flow pcu/h	Total Cap. veh/h	Prac. Deg. Satn xp	Prac. Spare Cap. %	Deg. Satn x
South: Leeds Parade								
2	T1	#	1	0	0	1889	0.98	****
3	R2	#	82	0	0	1753	0.98	1995
East: University								
4	L2	#	27	0	0	1511	0.80	4377
6	R2	#	1	0	0	56	0.80	4377
North: Leeds Parade								
7	L2	#	1	0	0	920	0.98	****
8	T1	#	1	0	0	920	0.98	****

\* Maximum degree of saturation

# Combined Movement Capacity parameters are shown for all Movement Classes.

### MOVEMENT PERFORMANCE

Mov ID	Turn	Total Delay (veh-h/h)	Total Delay (pers-h/h)	Aver. Delay (sec)	Eff. Stop Rate	Total Stops	Perf. Index	Tot.Trav. Distance (veh-km/h)	Tot.Trav. Time (veh-h/h)	Aver. Speed (km/h)
South: Leeds Parade										
2	T1	0.00	0.00	0.0	0.00	0.0	0.01	0.6	0.0	50.0
3	R2	0.01	0.01	8.0	0.67	54.9	1.44	47.6	1.1	42.7
East: University										
4	L2	0.00	0.00	7.5	0.63	17.1	0.52	15.8	0.4	43.1
6	R2	0.00	0.00	7.5	0.63	0.6	0.07	0.6	0.0	43.1
North: Leeds Parade										
7	L2	0.00	0.00	3.7	0.40	0.4	0.02	0.6	0.0	46.3
8	T1	0.00	0.00	3.7	0.40	0.4	0.01	0.6	0.0	46.3

[Go to Table Links \(Top\)](#)

## Fuel Consumption, Emissions and Cost

Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### FUEL CONSUMPTION, EMISSIONS AND COST (TOTAL)

Mov ID	Turn	Cost Total \$/h	Fuel Total L/h	CO2 Total kg/h	CO Total kg/h	HC Total kg/h	NOX Total kg/h
South: Leeds Parade							
2	T1	0.41	0.0	0.1	0.00	0.000	0.000
3	R2	39.01	3.1	7.4	0.02	0.003	0.018
		39.42	3.2	7.5	0.02	0.003	0.018
East: University							
4	L2	12.78	1.0	2.4	0.01	0.001	0.006
6	R2	0.47	0.0	0.1	0.00	0.000	0.000
		13.25	1.1	2.5	0.01	0.001	0.006
North: Leeds Parade							
7	L2	0.44	0.0	0.1	0.00	0.000	0.000
8	T1	0.44	0.0	0.1	0.00	0.000	0.000
		0.88	0.1	0.2	0.00	0.000	0.000
INTERSECTION:		53.55	4.3	10.2	0.03	0.004	0.025

### FUEL CONSUMPTION, EMISSIONS AND COST (RATE)

Mov ID	Turn	Cost Rate \$/km	Fuel Rate L/100km	CO2 Rate g/km	CO Rate g/km	HC Rate g/km	NOX Rate g/km
South: Leeds Parade							
2	T1	0.70	6.1	144.0	0.43	0.051	0.352
3	R2	0.82	6.6	156.5	0.48	0.061	0.381
		0.82	6.6	156.3	0.48	0.061	0.380
East: University							
4	L2	0.81	6.6	155.1	0.48	0.060	0.375
6	R2	0.81	6.6	155.1	0.48	0.060	0.375
		0.81	6.6	155.1	0.48	0.060	0.375
North: Leeds Parade							
7	L2	0.76	6.3	149.5	0.46	0.056	0.364
8	T1	0.76	6.3	149.5	0.46	0.056	0.364
		0.76	6.3	149.5	0.46	0.056	0.364
INTERSECTION:		0.68	5.5	129.9	0.40	0.050	0.316

[Go to Table Links \(Top\)](#)

## Lanes

### Lane Performance and Capacity Information

Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

### LANE PERFORMANCE

						Q u e u e		
Lane	Flow	Cap	Deg.	Aver.	Eff.	95% Back		Lane
No.	veh/h	veh/h	Satn	Delay	Stop	-----		Length
			x	sec	Rate	veh	m	m
South: Leeds Parade								
1	1	1889	0.001	0.0	0.00			500.0
2	82	1753	0.047	8.0	0.67	0.2	1.6	60.0T

-----								
East: University								
1	28	1567	0.018	7.5	0.63	0.1	0.5	140.0
-----								
North: Leeds Parade								
1	2	1840	0.001	3.7	0.40			500.0
-----								
T Short lane due to specification of Turn Bay								

## LANE FLOW AND CAPACITY INFORMATION

-----					
Lane No.	Total Arv Flow (veh/h)	Min Cap veh/h	Tot Cap veh/h	Deg. Satn x	Lane Util %
-----					
South: Leeds Parade					
1	1	1	1889	0.001	100
2	82	6	1753	0.047	100
-----					
East: University					
1	28	6	1567	0.018	100
-----					
North: Leeds Parade					
1	2	2	1840	0.001	100
-----					

The capacity value for priority and continuous movements is obtained by adjusting the basic saturation flow for heavy vehicle and turning vehicle effects. Saturation flow scale applies if specified.

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## Lane Delays

Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE DELAYS

						Delay (seconds/veh)					
Lane No.	Deg. Satn	Prog. Factor	Stop-1st	line 2nd	Delay Total	Acc. Dec. dn	Queuing Total dq	MvUp dqm	Stopd (Idle) di	Geom dig	Control dic
	x		d1	d2	dSL						
-----											
South: 1	Leeds 0.001	Parade		0.0					0.0	0.0	
2	0.047	1.000	0.0	0.0	0.0	0.1	0.0	0.0	0.0	8.0	8.0
-----											
East: 1	University 0.018		0.0	0.0	0.0	0.0	0.0	0.0	0.0	7.5	7.5
-----											
North: 1	Leeds 0.001	Parade		0.0					3.7	3.7	
-----											

SIDRA Standard Delay Model is used. Control Delay is the sum of Stop-line Delay and Geometric Delay.

dSL: Stop-line delay (=d1+d2)

dn: Average stop-start delay for all vehicles queued and unqueued

dq: Queuing delay (the part of the stop-line delay that includes stopped delay and queue move-up delay)

dqm: Queue move-up delay

di: Stopped delay (stopped (idling) time at near-zero speed)

dig: Geometric delay

dic: Control delay

## LANE DELAY PERCENTILES

-----								
Lane No.	Deg. Satn x	Percentile Delay						
		50%	70%	85%	90%	95%	98%	100%
-----								
South: Leeds Parade								
1	NA - Continuous Movement							
2	0.047	8.0	8.0	8.0	8.0	8.0	8.0	8.0
-----								
East: University								
1	0.018	7.5	7.5	7.5	7.5	7.5	7.6	7.6
-----								
North: Leeds Parade								
1	NA - Continuous Movement							
-----								

[Go to Table Links \(Top\)](#)

## Lane Queues

Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE QUEUES (VEHICLES)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (veh)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Nc	Queue 95%
	x		No	Nb1	Nb2	Nb	95%					
South: Leeds Parade												
2	0.047	1.000	0.0	0.1	0.0	0.1	0.2	0.01	0.0	100.0	0.0	0.0
East: University												
1	0.018	1.000	0.0	0.0	0.0	0.0	0.1	0.00	0.0	100.0	0.0	0.0
North: Leeds Parade												

## LANE QUEUES (DISTANCE)

Lane No.	Deg. Satn	Prog. Factor	Ovrfl. Queue No	Back of Queue (m)				Queue Stor. Ratio	Prob. Block %	P'ile Block %	Cyc-Av. Queue
	x			Nb1	Nb2	Nb	95%				Nc
											95%
South: Leeds Parade											
2	0.047	1.000	0.0	0.6	0.0	0.6	1.6	0.01	0.0	100.0	0.0
East: University											
1	0.018	1.000	0.0	0.2	0.0	0.2	0.5	0.00	0.0	100.0	0.0
North: Leeds Parade											

[Go to Table Links \(Top\)](#)

## Lane Queue Percentiles

Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE QUEUE PERCENTILES (VEHICLES)

Lane No.	Deg. Satn x	Percentile Back of Queue (veh)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade								
2	0.047	0.1	0.1	0.2	0.2	0.2	0.2	0.3
East: University								
1	0.018	0.0	0.0	0.1	0.1	0.1	0.1	0.1
North: Leeds Parade								

## LANE QUEUE PERCENTILES (DISTANCE)

Lane No.	Deg. Satn x	Percentile Back of Queue (metres)						
		50%	70%	85%	90%	95%	98%	100%
South: Leeds Parade								
2	0.047	0.6	0.8	1.2	1.4	1.6	1.8	1.9
East: University								
1	0.018	0.2	0.3	0.4	0.4	0.5	0.6	0.6
North: Leeds Parade								

[Go to Table Links \(Top\)](#)

## Lane Stops

### Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

Lane No.	Deg. Satn x	Prog. Factor	-- Effective		Stop Geom. hig	Rate -- Overall h	Queue Total Stops H	Total Move-up Rate hqm	Queue Move-ups Hqm	Prop. Queued pq
South: Leeds Parade										
1	0.001	1.000			0.00	0.00	0.0			
2	0.047	1.000	0.00	0.00	0.67	0.67	54.9	0.00	0.0	0.02
East: University										
1	0.018	1.000	0.00	0.00	0.63	0.63	17.8	0.00	0.0	0.01
North: Leeds Parade										
1	0.001	1.000			0.40	0.40	0.8			
hig is the average value for all movements in a shared lane										
hqm is average queue move-up rate for all vehicles queued and unqueued										

[Go to Table Links \(Top\)](#)

## Flow Rates

### Origin-Destination Flow Rates (Total)

#### Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### TOTAL FLOW RATES (ALL MOVEMENT CLASSES)

From SOUTH To:		N	E	
Turn:		T1	R2	TOT
Flow Rate		1.0	82.0	83.0
%HV (all designations)		5.0	5.0	5.0
From EAST To:		S	N	
Turn:		L2	R2	TOT
Flow Rate		27.0	1.0	28.0
%HV (all designations)		5.0	5.0	5.0
From NORTH To:		E	S	
Turn:		L2	T1	TOT
Flow Rate		1.0	1.0	2.0
%HV (all designations)		5.0	5.0	5.0

[Go to Table Links \(Top\)](#)

### Origin-Destination Flow Rates by Movement Class

#### Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

#### FLOW RATES FOR Light Vehicles

From SOUTH To:		N	E	
Turn:		T1	R2	TOT
Flow Rate - Veh		0.9	77.9	78.8
Mov Class %		95.0	95.0	95.0
Flow Scale - Fixed		1.00	1.00	
Flow Scale - Var		1.00	1.00	
Peak Flow Factor		1.00	1.00	
From EAST To:		S	N	
Turn:		L2	R2	TOT
Flow Rate - Veh		25.6	0.9	26.6
Mov Class %		95.0	95.0	95.0
Flow Scale - Fixed		1.00	1.00	
Flow Scale - Var		1.00	1.00	
Peak Flow Factor		1.00	1.00	

From NORTH To:	E	S	
Turn:	L2	T1	TOT
Flow Rate - Veh	0.9	0.9	1.9
Mov Class %	95.0	95.0	95.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

## FLOW RATES FOR Heavy Vehicles

From SOUTH To:	N	E	
Turn:	T1	R2	TOT
Flow Rate - Veh	0.1	4.1	4.2
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

From EAST To:	S	N	
Turn:	L2	R2	TOT
Flow Rate - Veh	1.4	0.1	1.4
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

From NORTH To:	E	S	
Turn:	L2	T1	TOT
Flow Rate - Veh	0.1	0.1	0.1
Mov Class %	5.0	5.0	5.0
Flow Scale - Fixed	1.00	1.00	
Flow Scale - Var	1.00	1.00	
Peak Flow Factor	1.00	1.00	

[Go to Table Links \(Top\)](#)

## Lane Flow Rates

Site:Uni existing AM

Intersection ID: 1  
Give-Way Sign Controlled Intersection

## LANE FLOW RATES AT STOP LINE

From SOUTH To:	N	E	
Turn:	T1	R2	TOT
Lane 1			
LV	0.9	*	0.9
HV	0.1	*	0.1
Total	1.0	*	1.0

Lane 2			
LV	*	77.9	77.9
HV	*	4.1	4.1
Total	*	82.0	82.0

Approach	1.0	82.0	83.0
----------	-----	------	------

From EAST To:	S	N	
Turn:	L2	R2	TOT

Lane 1			
LV	25.6	0.9	26.6
HV	1.4	0.1	1.4
Total	27.0	1.0	28.0

Approach	27.0	1.0	28.0
----------	------	-----	------

From NORTH To:	E	S	
Turn:	L2	T1	TOT

Lane 1			
LV	0.9	0.9	1.9
HV	0.1	0.1	0.1
Total	1.0	1.0	2.0

Approach	1.0	1.0	2.0
----------	-----	-----	-----

\* Movement not allocated to the lane

## EXIT LANE FLOW RATES

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	26.6	1.4
Total	26.6	1.4
Exit: EAST		
Lane: 1	78.8	4.2
Total	78.8	4.2
Exit: NORTH		
Lane: 1	1.9	0.1
Total	1.9	0.1

\* Movement not allocated to the lane

## DOWNSTREAM LANE FLOW RATES FOR EXIT ROADS

Movement Class:	LV	HV
Exit: SOUTH		
Lane: 1	26.6	1.4
Total	26.6	1.4
Exit: EAST		
Lane: 1	78.8	4.2
Total	78.8	4.2
Exit: NORTH		
Lane: 1	1.9	0.1
Total	1.9	0.1

\* Movement not allocated to the lane

Unit Time for Volumes = 60 minutes  
 Peak Flow Period = 30 minutes  
 Flow Rates include effects of Flow Scale and Peak Flow Factor

[Go to Table Links \(Top\)](#)

## Other

## Model Settings Summary

Site:Uni existing AM

Intersection ID: 1  
 Give-Way Sign Controlled Intersection

\* Basic Parameters:  
 Intersection Type: Unsignalised - Give Way  
 Driving on the left-hand side of the road  
 Input data specified in Metric units  
 Model Defaults: New South Wales  
 Peak Flow Period (for performance): 30 minutes  
 Unit time (for volumes): 60 minutes.  
 SIDRA Standard Delay model used  
 SIDRA Standard Queue model used  
 Level of Service based on: Delay (RTA NSW)  
 Queue percentile: 95%

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## Diagnostics

Site:Uni existing AM

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Processed: 2 February 2016 8:59:22 AM  
 SIDRA INTERSECTION 6.0.1.3703

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**SIDRA**  
**INTERSECTION 6**